

NEW YORK AND NEW JERSEY HARBOR DEEPENING CHANNEL IMPROVEMENTS NAVIGATION STUDY

DRAFT INTEGRATED FEASIBILITY REPORT & ENVIRONMENTAL ASSESSMENT

APPENDIX B2: GEOTECHNICAL ANALYSIS

TABLE OF CONTENTS

1. INTRODUCTION	1
2. GEOLOGICAL INFORMATION	2
 2.1. BEDROCK LITHOLOGY AND TOP CONTOUR MAP OF ROCK 2.2. PLEISTOCENE SEDIMENTS 2.3. SAND/GRAVEL SEDIMENTS (PLEISTOCENE-HOLOCENE) 2.4. HOLOCENE (RECENT) SEDIMENTS 	2 3 4 4
3. DATA REVIEW AND ANALYSIS METHODOLOGY	5
 3.1. REVIEW AND INVENTORY OF EXISTING SUBSURFACE DATA	5 5 6 7
4. DREDGEABILITY AND SLOPE ANALYSES	8
 4.1. ANCHORAGE CHANNEL (AN-1)	
4.2. PORT JERSEY CHANNEL (PJ-1) 4.2.1. Stratification 4.2.2. Dredgeability 4.2.3. Slope Stability	10 10 10 11
4.3. KILL VAN KULL CHANNEL (KVK-1)	
 4.4. NEWARK BAY (NWK-1)	15 15 16 17
 4.5. NEWARK BAY (NWK-2)	20

Attachments

Attachment #1	Figures
Attachment #2	SPT Boring Logs
Attachment #3	Slope Analyses

1. Introduction

New York-New Jersey Harbor consists of numerous bays, rivers, and channels of complex shape that are connected to the Hudson River. Some channels and bays within the New York -New Jersey Harbor will go through widening, deepening, and bend easing to improve navigation efficiencies within the New York and New Jersey Harbor and allow passage of Maersk Triple E Ultra Large Container Vessel Class (Triple E) as design vessel for this study. The Feasibility Study of New York and New Jersey Harbor Deepening Channel Improvement is an evaluation for channel widening, deepening, bend easing, and related improvements throughout the Ports of New York and New Jersey. The study areas include Anchorage, Port Jersey, Kill Van Kull, Arthur Kill, and Newark Bay reaches (Figure 1).

To perform the economical analysis of widening, deepening, and bend easing the channels, a number of factors will need to be considered. These factors include the physical nature of the material to be removed, how much material will be removed, appropriate methods of removal, where to dispose of dredged material, and what effects the project will have on nearby properties. In order to provide proper solutions for these factors, relatively detailed knowledges of the subsurface are required as follows:

- mapping the top of bedrock and delineate where it will occur along the proposed channel improvements so that volumetric calculations can be performed;
- mapping the thickness of contaminated material throughout the port so that volumetric calculations of contaminated material can be performed;
- other mappings such as Top of Pleistocene and sand/gravel to help define the dredgeability and disposal options for the dredged materials in the various reaches;
- defining profiles and cross sections to illustrate the aerial distribution of the various soil and rock types that will be encountered along each reach. Based on the profiles and cross sections, a specific course of action such as blasting or dredging with a specific type of dredge is determined;
- defining the characteristics (soil or rock classification, density, unit weight, compressive strength etc.) of various soil and rock types to be encountered along each reach so that dredgeability analysis and slope stability analysis can be performed;
- determining which dredging equipment that might be best suited for each stratigraphic reach using available computer programs and dredging records;
- analyzing the stability of the channel side slopes; and
- evaluating the potential effects that blasting may have on nearby structures.

Recent economical analysis shows deepening the pathways by 4 feet (to fifty-four feet below mean

lower low water (-54' MLLW)) and by 5 feet (to fifty-five feet below mean lower low water (-55' MLLW)) have considerable net benefits. Hence, in this study, dredgeability and slope stability analyses for project areas were performed on the basis of both 4 feet and 5 feet deepenings.

2. Geological Information

2.1. Bedrock Lithology and Top Contour Map of Rock

The locations and types of rock which are encountered if the channels are deepened to proposed deepening depths of fifty-four feet below mean lower low water (-54' MLLW) and fifty-five feet below mean lower low water (-55' MLLW) are shown in Figure 2. The top contour map of rock can be employed to determine the volume of rock that will require removal. The location, type and volumes of rock can be used for blasting analysis and cost analysis.

Bedrocks in the Kill Van Kull are composed of Serpentinite as metamorphic rocks in the Constable Hook reaches, Sandstone as a sedimentary rock in the Bergen Point reach, Diabase as an igneous rock from Bergen Point to the eastern part of Newark Bay, and Shale as a sedimentary rock with the arkosic sandstone member at Shooters Island. Throughout the Kill Van Kull, previously blasted, fractured, or moderately weathered bedrock underlies the channel.

Bedrocks in the Newark Bay consist of mainly Sandstone with shale member on the west, and mainly Diabase with Sandstone, Serpentinite, and Shale on the east. The Diabase, Sandstone and Shale are members of the Brunswick Formation (Jurassic) of the Newark Basin Physiographic Province.

Bedrocks in the Port Jersey Channel is mainly Schist. The east of the Port Jersey Channel is known to be in thrust-fault contact with Serpentinite associated with the Hartland Formation. Bedrock was not encountered above the projected project depths (-54' MLLW and -55' MLLW) in the Anchorage and Port Jersey channels.

Poorly consolidated sandstone and shale of the Atlantic Coastal Plain Physiographic Province (Tertiary-Cretaceous) underlies Ambrose Channel but were not encountered within the depth of El. -75 MLLW.

2.2. Pleistocene Sediments

The bedrock complex is overlain by Pleistocene glacial sediments that range in size from silt and clay to large cobbles and boulders. In most cases glaciation subdued the terrain by leveling the higher peaks and filling the valleys with glacial till (ground moraine). However, in some cases, low relief, elongated hills were formed by debris pushed in front (terminal moraine) or to the sides (lateral moraines) of the advancing glacier. A terminal moraine exists along the south edge of Staten Island up to the Narrows, and then appears again in the Bay Ridge section of Brooklyn. The terminal moraine continues out on Long Island to Nassau County where it splits into two separate terminal moraines that continue out to Montauk Point and Orient Point. The southern terminal moraine is known as the Ronkonkoma and the northern as the Harbor Hill. Adjacent to the Narrows, the Harbor Hill terminal moraine overlies the Ronkonkoma.

Southeast of the terminal moraine are outwash deposits of sand and gravel that extend under Lower New York Bay. These fluvial deposits are the result of ice melt water washing soils out beyond the terminal moraine. Behind the terminal moraine is a ground surface moraine deposit consisting of an unsorted mixture of sand, silt, clay, gravel, cobbles and boulders. These materials were picked up by the advancing ice and dropped as the ice melted. Scattered within this ground moraine are fluvial deposits, such as eskers and kames, which were deposited by concentrated ice melt water that flowed in and under the ice.

As the glacier receded, the terminal moraine became a natural dam that impounded the glacier melt water and formed a number of large lakes. Varved silts and clays were deposited within these lakes. These varved deposits which overlie ground moraines are sometimes as thick as one hundred feet (100'). Sands were deposited on top of the varved silt and clay when the lakes became shallow. Eventually the terminal moraine was breached, and the glacial lakes were drained. Subsequent to the draining of these glacial lakes, outwash plains composed of alluvial sand deposits were formed along the perimeter of the lakebeds.

Figure 3 shows a top contour map of Pleistocene. The map defines where the dense (more difficult to dredge) Pleistocene Sediments will be encountered. It should be noted that Pleistocene sediments are normally red to brown, rarely contain shells and are relatively dense when compared to Holocene (Recent) sediments, which are normally gray to black and frequently contain shells. Although there are significant differences between Pleistocene and Holocene (Recent) sediments it is not always easy to define the top of the Pleistocene. Based on N values and soil descriptions of soil borings, it is anticipated that most of sediments in Anchorage, Port Jersey, Kill Van Kull, and Newark bay reaches will be Pleistocene. Top of Pleistocene contour map can be considered a Paleo-structure or Paleo-geographic map. It is known that the top of the Pleistocene normally occurs at an elevation of approximately 20 feet below mean lower low water (-20' MLLW) along the flats adjacent to channels.

2.3. Sand/Gravel Sediments (Pleistocene-Holocene)

Map of sand and gravel-sized sediments above sixty-feet below mean lower low water (-60' MLLW) is shown on Figure 4. The map can be used to determine where the dredged material will be predominantly sand and gravel, silt and clay or some combination. The map should be used in conjunction with the Top of Pleistocene Map, Isopach Maps, as well as profiles and cross-sections to help define the most suitable contract limits, possible dredge types, and disposal location. All these factors are employed in the cost analysis.

2.4. Holocene (Recent) Sediments

It is known that during the last 20,000 years, sea level has risen as much as three hundred feet (300') and continues to rise at a rate of one to two feet (1'-2') per 100 years. As a result, the project area that was high and dry during the Pleistocene has become a tide-dominated estuary. A blanket of Holocene (Recent) sediments ranging from poorly graded sand, silty sand, slightly organic silt, and occasionally peat has been deposited. The thickness of the Holocene section is a few feet in the flats adjacent to the navigation channels.

The upper portion of the Holocene section often emits a petroleum-like odor and contains man made debris including toxic chemicals and heavy metals. One objective of this study was to estimate the aerial distribution (location and thickness) of the contaminated material so that volumes could be approximated. This was accomplished through the use of isopach (thickness) maps. The thickness of contaminated material above sixty feet below mean lower low water (-60' MLLW) was determined in those borings in which it was detected. The thickness of contaminated interval was estimated in each boring from physical characteristics (moisture content, color, odor, and soil density), scanning methods (UV fluorescence and photo-ionization) and direct testing (total petroleum hydrocarbons). The isopach maps shown in Figure 5 show the location and aerial distribution of contaminated sediments that will require removal and disposal as the channels are deepened. In this study, soft and loose soils were assumed to be an indicative of Holocene deposits, namely, not HARS suitable.

3. Data Review and Analysis Methodology

3.1. Review and Inventory of Existing Subsurface Data

A review of previous Corps of Engineer reports and dredging contracts was conducted to obtain existing subsurface information. Other public agencies and private sources were contacted to determine the availability of additional subsurface data. As few soil testing borings were located within limits of proposed reaches, soil testing borings located outside of limits of proposed Reaches also were considered to obtain existing subsurface information of proposed reaches. The distribution, depth, and quality of the available data was reviewed to determine the need for additional subsurface. It was shown through the review that some of the channels such as Kill Van Kull, and Anchorage would require very little additional data whereas other channels including Newark Bay and Port Jersey would require more additional subsurface data due to lack of existing subsurface data.

3.2. Field Testing and Subsurface Data

Standard Penetration Test (SPT) methods have been applied to collect continuous soil samples throughout the proposed reaches. Representative soil and rock samples were selected for testing from both SPT borings and vibracores to define the geotechnical properties of the soil necessary for slope stability analysis, dredging analysis and blasting analysis and to employ possible low cost and efficient techniques to determine the thickness of the contaminated interval. Geotechnical testing included grain size distribution, moisture content, unit weight, specific gravity, liquid and plastic limits. Multiple undisturbed (Shelby Tube) samples were recovered from each of the major silt and clay layers. Triaxial tests were performed on the undisturbed samples to determine shear strength, cohesion and other factors necessary for slope stability. Rock cores were recovered in those areas where shallow (<65'from MLLW) bedrock was encountered. Cores were described in detail and rock quality designation (RQD) was determined. Point load tests were conducted on representative core samples.

The thickness of the contaminated interval was determined by use of ultra-violet fluorescence, photo ionization, total petroleum hydrocarbons and moisture content.

3.3. Soil Profiles and Cross-Sections

In the subsequent subsections, the individual reaches are described in detail through the use of soil profiles and cross-sections. The profiles were employed to illustrate the aerial extent of the various soil and rock types that will be encountered within reaches. Soils that have similar characteristics (i.e. Silty Sand and Silty Gravel) or that are difficult to differentiate in the field (i.e. Silty Clay or Clayey Silt) were grouped together to define stratigraphic units. Soil color was employed as

another factor in determining the stratigraphic units illustrated. Pleistocene sediments are commonly reddish-brown whereas the Holocene sediments are frequently gray, tan or black. Although a stratigraphic unit is, for instance, called Holocene Silt and Clay, it may contain minor sand or gravel sub-units.

Cross sections show the stratigraphy of soil and rock types that are encountered along the middle of reaches. The soil types and their properties such as unit weight, cohesion, and friction angle are very significant factors to be considered in determining side slope stability.

3.4. Dredgeability Analysis

Guides of program DREDGABL developed by WES was used to aid Engineers in determining the suitability of various sediments to dredging. The guides lead engineers to determine if a given sediment type can be dredged and what dredges are most appropriate for the dredging procedure. The guides require the soil type, rock type, fineness (grain size), consistency (i.e., soft, medium, stiff, or hard), and in-situ compactness (i.e., loose, medium, or dense). For compressible sediments (both Organic and Inorganic Clay and Silt), soil type described by using the Unified Soil Classification System (USCS) and soil consistency are needed to perform the dredgeability analysis. For coarser sediments (Sand and Gravel), specific USCS soil type and compactness are needed. Additionally, the guides provide dredgeability information for rock and shale fragments, cemented soils, shells and debris.

Table 1 is a summary of the general dredging characteristics of the overall dredge types. Specific dredges are listed for each reach. Before a final dredge type is selected, coordination with other agencies must be made. Some dredge types are not permitted because of environmental concerns, cost or availability. Final selection of the dredge type to be used lies with the Contractor in coordination with the USACE and other agencies.

Dredge Type	% Solids in Slurry By Weight	Turbidity	Open Water Operation	Range of Production (cu.yd. /hr.)	Dredging Depth Limits (ft)	Wave Height Limits (ft)	Limiting Currents
Hopper Dredges	10 - 20	High	Yes	500 - 2000	< 80	< 7	7 Knots
Mechanical Dredges	In Situ	Average	Yes	30 - 500	< 100	< 3	3 Knots
Pipeline Dredges	10 - 20	Average	Depends On Type	25 - 10,000	< 14	< 3	3 - 7 Knots

Table 1: Summary of Dredge Operating Characteristics

Each reach can be considered either "soft" or "moderate to hard" as a reference to the ease of dredging. Overall, Ambrose, Anchorage, and Port Jersey Channel can be considered "soft" because the sediments are relatively loose sands or soft to very soft silts and clays and should be easily dredged. Channels such as Kill Van Kull, Newark Bay, and Port Jersey can be considered "moderate to hard" either because the sediments are dense to very dense or rock is present; these channels would require additional efforts during dredging including rock blasting. According to previous experiences, rocks with unconfined compressive strengths less than 7,000 psi or RQD values less than 30% is considered dredgeable.

3.5. Slope Stability Analysis

It is necessary to determine the stability of the channel side slopes for the following reasons:

- If there are structures close to the top of the slope and failure occurs, the structural stability of the structure is diminished, and very significant damage may occur.
- If reach side slopes are not stable, then failure will result in significant shoaling. Frequent expensive maintenance including dredging will be required.
- Proper measures to unstable slopes can be taken into consideration. The measures can be simulated to verify their effectiveness.
- After estimating approximate full extents of unstable slopes and determining proper measures to unstable slopes, total cost for stabilization of unstable slopes can be approximated.

Slope stability analysis was performed using the computer program SLOPE/W developed by Geo-Slope International, Ltd. The program requires input including data such as channel slope profiles, stratigraphy, and soil properties. The Spencer's method was used to obtain the lowest factor of safety for the governing cross section of each slope. Circular and non-circular potential failure surfaces were searched with optimization.

Side slopes of each reach in non-rock material were constructed and have been maintained at a 1 (rise) on 3 (run) ratio. These slopes are very similar to those that existed along the natural channel prior to dredging. It is known that these side slopes have been proven to be stable near critical structures for all types of soils within the channel. However, even slopes with 1 on 3 may not be stable especially for soft soils such organic silts, peat, and loose sands. Excessive sloughing of these types of soils may occur causing more frequent maintenance dredging. Hence, while in view of these factors, a side slope of 1 on 3 is considered where possible, resulting unstable slopes should be stabilized by low cost measures. The results of slope stability based on a 1 on 3 side slope is summarized for each channel evaluated.

The existing channel side slopes in rock areas were constructed at a 1 (rise) on 1 (run) ratio. This slope has proven to be stable, especially near structures. The 1 on 1 side slopes were basically formed from the blasting. The explosives in the blast holes break and push the rock upward in a shape of a 45 degree angle cone. These 1 on 1 side slopes in rock are relatively easy for a contractor to dredge after blasting. In view of these factors a side slope of 1 on 1 is recommended where possible in rock areas. In areas where a steeper slope is required, more detailed analysis should be performed. Factors such as the orientation (strike and dip) of fractures and bedding planes should be determined.

4. Dredgeability and Slope Analyses

4.1. Anchorage Channel (AN-1)

4.1.1. Stratification

Anchorage Channel is the primary channel in the Upper Bay. Reach AN-1 is located at the south end of the Anchorage Channel. Soil test borings performed at this reach are displayed in Figure 6. The stratification of Reach AN-1 was determined based on types and consistencies of each material encountered in the soil test borings. The stratification of Reach AN-1 is shown in Figure 11. The profile indicates that the Reach AN-1 is primarily composed of Holocene (Recent) Silt. Based on proposed deepening depths of fifty-four feet below mean lower low water (-54 MLLW) and fifty-five feet below mean lower low water (-55' MLLW), Silt can easily be determined as a material

to be removed when the reach are deepened.

4.1.2. Dredgeability

For the purpose of DREDGABL analysis, the Silt in Reach AN-1 was classified as either OL or OH with very soft to soft consistencies and medium to high plasticity. These criteria were determined from a review of the boring logs in the reach. The Silts of Reach AN-1 have excavation characteristics classified as "good" for clamshell dredge, "very good to good" for backhoe dredge, and "very good" for cutter dredge as shown in Table 2.

Material	Dredge Type					
Туре	Clamshell	Backhoe	Cutter	Blast		
Silt	Good	Very Good to Good	Very Good	NA		

Table 2: Dredgeability of Materials for Reach AN-1

When factors such as dredging records, contamination, disposal, and availability of dredging equipment are considered, an environmentally friendly clamshell dredge would appear to be the most suitable equipment to dredge Reach AN-1.

4.1.3. Slope Stability

Soil test boring location RHF98-25 was used to estimate soil strata and soil properties for analyses of slope stability as overall, the boring can provide whole soil stratification which is useful to understand stabilities of slope proposed for four feet and five feet deepenings. The soil properties of soil strata of the boring are shown below:

Table 3: Soil Pro	perties of Soi	l Strata at	Reach AN-1

Boring No.	Material Type	Top Elevation of Stratum (ft)	Representative N-Value (blows/ft) /RQD (%)	Unit Weight (pcf)	Cohesion (psf)	Friction Angle (Deg.)
DUE09 25	Silt	-45.1	0	90	50	0
KHF98-25	Silt	-53.0	0	100	100	0

Using the above soil properties, circular and non-circular potential failure surfaces were searched by optimized grid-radius, entry-exit, and block analyses. As a result of simulations, the lowest safety factors of slope analyses for four feet and five feet deepenings at this reach equals 1.71 and 1.51, respectively; the slope stability criteria with the minimum factor of safety of 1.5 was satisfied. The summary of slope stability analyses at Reach AN-1 is shown in Table 4.

Boring No.	Factors of S Deepo	Safety of 4' ening	Factors of Safety of 5' Deepening		
	Min	Max	Min	Max	
RHF98-25	1.711	1.794	1.505	1.567	

Table 4: Summary of Slope Analyses at Reach AN-1

4.2. Port Jersey Channel (PJ-1)

4.2.1. Stratification

The Port Jersey Channel is located in Bayonne, New Jersey between the Military Ocean Terminal on the south and the Global Marine Terminal and the Northeast Automobile Terminal on the north. Reach PJ-1 is a triangle-shaped reach located at the entry of the Port Jersey Channel from Anchorage Channel. Soil test borings performed at this reach are displayed on Figure 7. The stratification of Reach PJ-1 was determined based on material types and material consistencies of the soil test borings. Reach PJ-1 mainly consists of:

- Holocene (Recent) sediments: soft black, grey, and dark grey Silt (OL or OH) and underlying Silty Clay (CL, OH or MH); and

- Pleistocene age sediments: a dense to very dense, fine to medium grained, red-brown Sand (SP-SM) with lenses of stiff Silt and Clay and occasional Gravel.

The stratification of Reach PJ-1 is shown in Figure 12.

4.2.2. Dredgeability

As the other sediments account for a limited amount of the total volume, Silt (OL or OH) and Silty Clay (CL, OH or MH) were analyzed as a main material to be dredged in Reach PJ-1. Based upon the results from the analysis, the Silt and Silty Clay of Reach PJ-1 have excavation characteristics classified as "good" for clamshell dredge, "very good to good" for backhoe dredge, and "very

good" for cutter dredge. The Sand (SP-SM) of Reach PJ-1 is classified as "good to fair" for clamshell dredge, "good" for backhoe dredge, and "very good to good" for cutter dredge as shown in Table 5.

Material	Dredge Type					
Туре	Clamshell	Backhoe	Cutter	Blast		
Silt & Silty Clay	Good	Very Good to Good	Very Good	NA		
Sand	Good to Fair	Good	Very Good to Good	NA		

Table 5: Dredgeability of Materials for Reach PJ-1

Consistencies of Silts and Silty Clays were classified as very soft to soft and the consistencies of Sands were classified as being fine to medium grained with medium density. Based upon previous analyses the Recent Silt and Silty Clay sediments can be dredged by all dredges being used for comparison, therefore, only the Sand was used for determining the suitability of the selected dredge types. All dredge types are proved to be suitable for this reach with the cutter dredge having the best results. In view of dredging records, contamination, disposal, and availability of dredging equipment, a clamshell dredge will be suitable to dredge the Reach PJ-1.

4.2.3. Slope Stability

Soil test boring locations ANC98-81 and PJ-4 were used to estimate soil strata and soil properties for analyses of slope stability as overall, the borings can provide whole soil stratification which is useful to understand stabilities of slope proposed for four feet and five feet deepenings. The soil properties of soil strata of the borings are shown in Table 6.

Boring No.	Material Type	Top Elevation of Stratum (ft)	Representative N-Value (blows/ft) /RQD (%)	Unit Weight (pcf)	Cohesion (psf)	Friction Angle (Deg.)
	Silt	-21.0	0	90	50	0
ANC98-81	Silty Clay	-33.0	0	100	100	0
	Sand	-63.5	70	135	0	42
	Silt	-32.0	0	90	50	0
PJ-4	Silty Clay	-35.0	0	100	100	0
	Sand	-42.0	80	135	0	42
	Schist	-84.0	NA	155	5000	40

Table 6: Soil Properties of Soil Strata at Reach PJ-1

Using the above soil properties, circular and non-circular potential failure surfaces were searched by optimized grid-radius, entry-exit, and block analyses. As a result of simulations, the area of boring PJ-4 in the western portion of the Reach PJ-1 resulted in the lowest safety factors of slope analyses for four feet and five feet deepenings as 2.7 and 2.69, respectively. On the other hand, the area of boring ANC98-81 in the eastern portion of the Reach PJ-1 resulted in the lowest safety factors of slope analyses for four feet and five feet and five feet deepenings as about 0.70 and 0.69, respectively. The area of boring PJ-4 in the western portion of the Reach PJ-1 satisfies the slope stability criteria with the minimum factor of safety of 1.5; however, the area of boring ANC98-81 in the eastern portion of the Reach PJ-1 satisfies the slope stability criteria with the minimum factor of safety of 1.5; however, the area of boring ANC98-81 in the eastern portion of the Reach PJ-1 satisfies the slope stability criteria with the minimum factor of safety of 1.5; however, the area of boring ANC98-81 in the eastern portion of the Reach PJ-1 fails to meet the minimum factor of safety. The summary of slope stability analyses at Reach PJ-1 is shown in Table 7.

Boring No.	Factors of S Deep	Safety of 4' ening	Factors of Safety of 5' Deepening	
	Min	Max	Min	Max
ANC98-81	0.696	0.709	0.690	0.700
PJ-4	2.702	2.706	2.690	2.702

Table 7: Summary of Slope Analyses at Reach PJ-1

Critical slope failures in the area of boring ANC98-81 occur over the bottom of thick, soft consistency Silty Clay layer. The full extent of the soft Silt and Silty Clay should need to be further investigated to determine a proper type and the scope of application of mitigation measure to stabilize the slopes in the eastern portion of the Reach PJ-1.

4.3. Kill Van Kull Channel (KVK-1)

4.3.1. Stratification

The Kill Van Kull Channel is located north of Staten Island, New York and south of Bayonne, New Jersey. The channel connects Upper New York Harbor with the Arthur Kill Channel and Newark Bay Complex. Reach KVK-1 is a triangle-shaped reach located on the south of entry of Kill Van Kull from Ambrose Channel. Soil test borings performed at this reach are displayed on Figure 8. The stratification of Reach KVK-1 was determined based on material types and material consistencies of the soil test borings. The Reach KVK-1 mainly consists of:

- Holocene (Recent) sediments: soft black, grey, and dark grey Silt (OL or OH) and a layer of dense to loose consistency Sand (SP-SM);

- Pleistocene age sediments: dense to very dense Cobble and Boulder; and
- Serpentinite as a bedrock.

The stratification of Reach KVK-1 is illustrated in Figure 13. Based on proposed deepening depths of fifty-four feet below mean lower low water (-54 MLLW) and fifty-five feet below mean lower low water (-55' MLLW), Holocene (Recent) sediments will mainly be removed when the reach is deepened.

4.3.2. Dredgeability

Based upon the results from the analysis of dredgeability, the Silt of Reach KVK-1 has excavation characteristics classified as "good" for clamshell dredge, "good" for backhoe dredge, and "very good" for cutter dredge. Sand is classified as "very good to fair" for clamshell dredge, "very good to good" for backhoe dredge, and "very good" for cutter dredge. Cobble and Boulder are classified as "poor" for clamshell dredge, "fair to good" for backhoe dredge, and "good" for cutter dredge. Serpentinite rock is encountered below fifty feet below mean lower low water (-50 MLLW) in the western portion of Reach KVK-1. These dredgeabilities of each material are summarized in Table 8.

Matarial Trues	Dredge Type					
Material Type	Clamshell Backhoe		Cutter	Blast		
Silt	Good	Good	Very Good	NA		
Sand	Very Good to Fair	Very Good to Good	Very Good	NA		
Cobble/Boulder	Poor	Fair to Good	Good	NA		
Serpentinite	Very Poor	Poor	Poor	Good		

Table 8: Dredgeability of Materials for Reach KVK-1

Consistencies of Silts were classified as very soft to soft and the consistencies of Sands were classified as density to loose. Consistencies of Cobble and Bounder were classified as dense to very dense and RQD of the Serpentinite rock was estimated as 44%. Based upon previous analyses, the Recent Silt and Sand sediments can be dredged by all dredges. In view of dredging records, contamination, disposal, and availability of dredging equipment, a clamshell dredge will be suitable to dredge the Recent Silt and Sand sediments in the Reach KVK-1. Cobble and Boulder can be dredged by backhoe and cutter dredges. Serpentinite rock encountered below fifty feet below mean lower low water (-50 MLLW) in the western portion of Reach KVK-1 should be dredged to a limited extent to reach the proposed deepening depths. Hence, a heavy cutter dredge will be economical and suitable to dredge the limited extent of Serpentinite rock in the western portion of the Reach KVK-1.

4.3.3. Slope Stability

Soil test boring locations ANC98-31, ANC98-30, and KVK98-1A were used to estimate soil strata and soil properties for analyses of slope stability as overall the borings can provide whole soil stratification which is useful to understand stabilities of slope proposed for four feet and five feet deepenings. The soil properties of soil strata of the borings are shown in Table 9.

Boring No.	Material Type	Top Elevation of Stratum (ft)	Representative N-Value (blows/ft) /RQD (%)	Unit Weight (pcf)	Cohesion (psf)	Friction Angle (Deg.)
	Silt	-42.0	0	90	50	0
ANC98-31	Sand	-44.0	8	115	0	31
	Silt	-50.0	0	90	50	0
	Silt	-27.0	0	90	50	0
ANC98-30	Sand	-43.0	10	120	0	30
	Silt	-55.0	0	90	50	0
KVK98-1A	Silt	-22.8	0	90	50	0
	Sand	-39.0	4	110	0	28
	Sand	-43.0	18	120	0	32.5
	Serpentinite	-54.0	44%	155	5000	40

 Table 9: Soil Properties of Soil Strata at Reach KVK-1

Using the above soil properties, circular and non-circular potential failure surfaces were searched

by optimized grid-radius, entry-exit, and block analyses. As a result of simulations, the area of boring ANC98-31 in the eastern portion of the Reach KVK-1 resulted in the lowest safety factors of slope analyses for four feet and five feet deepenings as about 0.54 and 0.49, respectively. The area of boring ANC98-30 in the middle of the Reach KVK-1 resulted in the same lowest safety factor of slope analyses for four feet and five feet deepenings as about 1.15. In addition, the area of boring KVK98-1A in the middle portion of the Reach KVK-1 resulted in the same lowest safety factor of slope analyses for four feet and five feet deepenings as about 1.15. In addition, the area of boring KVK98-1A in the middle portion of the Reach KVK-1 resulted in the same lowest safety factor of slope analyses for four feet and five feet deepenings as about 1.14. Overall, most of the Reach KVK-1 look like not satisfying the slope stability criteria with the minimum factor of safety of 1.5. The summary of slope stability analyses at Reach KVK-1 is shown in Table 10.

Boring No.	Factors of Safety of 4' Deepening		Factors of Safety of 5' Deepening	
	Min	Max	Min	Max
ANC98-31	0.473	0.481	0.488	0.573
ANC98-30	1.148	1.159	1.148	1.159
KVK98-1A	1.144	1.170	1.144	1.170

Table 10: Summary of Slope Analyses at Reach KVK-1

It is anticipated that critical slope failures in the eastern portion of Reach KVK-1 including boring ANC98-31 will occur as mode of deep-seated slope failure due to very thick, soft consistency top and third layers of silt materials and relatively thin, medium consistency second sand layer. On the other hand, critical slope failures in the eastern and middle portions of Reach KVK-1 including borings ANC98-30, KVK98-1A, and KN-11 will occur as mode of local slope failure due to a relatively thick, medium to firm consistency second layer of sand in the middle of the reach and dense to very dense second layer of boulder/cobble in the eastern portion.

The full extent of the soft consistency silt should need to be further investigated to determine a proper type and the scope of application of mitigation measure to stabilize the slopes in the Reach KVK-1.

4.4. Newark Bay (NWK-1)

4.4.1. Stratification

Newark Bay is located at the north of the junction of the Kill Van Kull and Arthur Kill Channels

and extends northward to the New Jersey Turnpike Extension Bridge in Jersey City, New Jersey. Reach NWK-1 is a triangle-shaped strip located and extending from south to north on the eastern side of Newark Bay. Soil test borings performed at this reach are displayed in Figure 9. The proposed deepening depths for the reach are fifty-four feet below mean lower low water (-54 MLLW) and fifty-five feet below mean lower low water (-55' MLLW). The Reach NWK-1 mainly consists of:

- Holocene (Recent) sediments: soft black, grey, and dark grey Silt (OL or OH);

- Pleistocene age sediments: hard to medium-stiff consistency Clay (CL or CH), very dense to medium consistency Sand (SP), and very dense to medium consistency Gravel (GC-GM); and

- Bedrocks such as Diabase, Diorite, Serpentinite, Shale, and/or Sandstone.

The stratification of Reach NWK-1 is illustrated in Figure 14. Based on the proposed deepening depths of fifty-four feet below mean lower low water (-54 MLLW) and fifty-five feet below mean lower low water (-55' MLLW), considerable amounts of Holocene (Recent) and Pleistocene sediments will be removed when the reach are deepened.

4.4.2. Dredgeability

Based upon the results from the analysis of dredgeability, the Silt of Reach NWK-1 has excavation characteristics classified as "good" for clamshell dredge, "good" for backhoe dredge, and "very good" for cutter dredge. Clay is classified as "poor" for clamshell dredge, "fair to poor" for backhoe dredge, and "good" for cutter dredge. Sand is classified as "good to fair" for clamshell dredge, "very good to good" for backhoe dredge, and "very good to good" for cutter dredge, and "very good to good" for clamshell dredge, "fair to poor" for backhoe dredge, and "good" for backhoe dredge, and "very good to good" for clamshell dredge, "fair to poor" for backhoe dredge. Gravel is classified as "poor" for clamshell dredge, "fair to poor" for backhoe dredge, and "good" for cutter dredge. Gravel is classified as "poor" for clamshell dredge, "fair to poor" for backhoe dredge, and "good" for cutter dredge. Most of sub-crops of bedrocks including Diabase, Diorite, Serpentinite, Shale, and/or Sandstone are encountered below fifty feet below mean lower low water (-50 MLLW) in the most portions of Reach NWK-1 except for northern portion of the reach; sub-crops of bedrocks in the northern portion of the reach extend close to twenty feet below mean lower low water (-20 MLLW). The dredgeabilities of each material are summarized in the Table 11.

Matarial Type	Dredge Type				
wrateriai Type	Clamshell	Backhoe	Cutter	Blast	
Silt	Good	Good	Very Good	NA	
Clay	Poor	Fair to Poor	Good	NA	
Sand	Good to Fair	Very Good to Good	Very Good to Good	NA	
Gravel	Poor	Fair to Poor	Good	NA	
Bedrocks (Diabase, Diorite, Serpentinite, Shale, and/or Sandstone)	Very Poor	Poor	Poor	Good	

Table 11: Dredgeability of Materials for Reach NWK-1

Consistencies of Silts were classified as very soft to soft and the consistencies of Clays were classified as hard to medium-stiff. Consistencies of Sands and Gravels were classified as very dense to medium, and RQDs of the bedrocks were estimated in the range of 25% to 85%. Based upon previous analyses, the Recent Silt sediments can be dredged by all dredge types. In view of dredging records, contamination, disposal, and availability of dredging equipment, a clamshell dredge will be suitable to dredge the Recent Silt sediments in the Reach NWK-1.

While Sand sediments also can be dredged by all dredges, Clay and Gravel sediments can be dredged only by cutter dredges. As the Sand sediments lies between the Clay and the Gravel sediments and Sand sediments can be dredged by all dredge types, it is efficient to determine a dredge type available for Clay and Gravel sediments. Hence, a cutter dredge will be suitable to dredge the Clay, Sand, and Gravel sediments.

As the most of bedrocks encountered in most portions of the Reach NWK-1 except for the northern portion are located below fifty feet below mean lower low water (-50 MLLW), a heavy cutter dredge will be economical and suitable to remove the limited extent of the bedrocks to reach the proposed deepening depths. However, it is anticipated that the significant amounts of bedrocks encountered in northern portion of the Reach NWK-1 will be in need of blasting to reach the proposed deepening depths. The full extent of the bedrocks in need of blasting in the northern portion should need to be further investigated to estimate more accurate costs and schedules.

4.4.3. Slope Stability

Soil test boring locations PA2-443, PA2-486, NB98-34, and NBN-01-SFI-2 were used to estimate

soil strata and soil properties for analyses of slope stability as overall, the borings can provide whole soil stratification which is useful to understand stabilities of slope proposed for four feet and five feet deepenings. The soil properties of soil strata of the borings are shown in Table 12.

Boring No.	Material Type	Top Elevation of Stratum (ft)	Representativ e N-Value (blows/ft) /RQD (%)	Unit Weight (pcf)	Cohesion (psf)	Frictio n Angle (Deg.)
	Silt	-11.5	0	90	50	0
	Clay	-41.5	5	110	630	0
PA2-443	Clay	-45.0	35	130	4375	0
	Clay	-64.0	10	120	1250	0
	Shale	-82.0	NA	155	5000	40
	Silt	-15.7	0	90	50	0
	Sand	-24.0	3	105	0	28
	Clay	-29.0	9	115	1125	0
NB98-34	Decompose d Shale	-44.0	75	135	0	37
	Shale, Serpentinit e, Diabase, and Sandstone	-49.0	NA	155	5000	40
	Silt	-6.4	0	90	50	0
NBN01-SFI-	Sand	-17.5	35	130	0	39.5
2	Clay	-21.5	35	130	4375	0
	Sandstone	-37.5	80%	155	5000	40
DA2 496	Silt	-16.5	0	90	50	0
	Clay	-35.0	30	125	3750	0
rA2-400	Sand	-48.0	55	135	0	41
	Diabase	-58.5	>70%	155	5000	40

Table 12: Soil Properties of Soil Strata at Reach NWK-1

Using the above soil properties, circular and non-circular potential failure surfaces were searched by optimized grid-radius, entry-exit, and block analyses. As a result of simulations, the area of boring PA2-443 in the northern end of the Reach NWK-1 resulted in the same lowest safety factor

of slope analyses for four feet and five feet deepenings as about 0.62. The area of boring NBN01-SFI-2 in the northern tenth of the Reach NWK-1 resulted in the lowest safety factors of slope analyses for four feet and five feet deepenings as about 1.68 and 1.67, respectively. The area of boring NB98-34 in the middle of the Reach NWK -1 resulted in the same lowest safety factor of slope analyses for four feet and five feet deepenings as about 1.94. The area of boring PA2-486 in the southern portion of the Reach NWK-1 resulted in the same lowest safety factor of slope analyses for four feet and five feet deepenings as about 1.00. Overall, the northern end and southern portion of the Reach NWK-1 look like not satisfying the slope stability criteria with the minimum factor of safety of 1.5, while the middle and northern tenth portions of the Reach NWK-1 meet the slope stability criteria. The summary of slope stability analyses at Reach NWK-1 is in Table 13.

Boring No.	Factors of Safety of 4' Deepening		Factors of Safety of 5' Deepening	
	Min	Max	Min	Max
PA2-443	0.617	0.645	0.617	0.637
NB98-34	1.944	1.953	1.944	1.962
NBN01-SFI-2	1.678	1.745	1.671	1.688
PA2-486	1.002	1.050	1.004	1.050

Table 13: Summary of Slope Analyses at Reach NWK-1

It is anticipated that critical slope failures in the northern end and southern portion of Reach NWK-1 including borings PA2-443 and PA2-486, respectively, will occur as mode of local slope failure due to thick, very soft consistency top layer of silt materials underlain by a relatively medium-stiff to hard consistency second layer of clay. On the other hand, it is anticipated that slope failures in the northern tenth and middle portions of Reach NWK-1 including borings NBN01-SFI-2 and NB98-34, respectively, would not occur due to a relatively thinner, soft consistency top layer of silt materials underlain by relatively denser sand layer.

The full extent of the very soft consistency silt should need to be further investigated to determine a proper type and the scope of application of mitigation measure to stabilize the slopes in the Reach NWK-1.

4.5. Newark Bay (NWK-2)

4.5.1. Stratification

Reach NWK-2 is located between South Elizabeth Channel and Arthur Kill Channel and on the southwestern side of Newark Bay. Soil test borings performed at this reach are displayed on Figure 10. The proposed deepening depths for the reach are fifty-four feet below mean lower low water (-54 MLLW) and fifty-five feet below mean lower low water (-55' MLLW). The Reach NWK-2 mainly consists of:

- Holocene (Recent) sediments: soft black, grey, and dark grey Silt (OL or OH);

- Pleistocene age sediments: hard to medium-stiff consistency Clay (CL or CH), and very dense to medium consistency Gravel (GC, GM, or GP); and

- Bedrocks such as Sandstone and/or Shale.

The stratification of Reach NWK-2 is shown on Figure 15. Based on proposed deepening depths of fifty-four feet below mean lower low water (-54 MLLW) and fifty-five feet below mean lower low water (-55' MLLW), a majority of Holocene (Recent) and Pleistocene sediments will be removed when the reach are deepened. A significant amount of bedrock also will be removed to reach the design deepening depths.

4.5.2. Dredgeability

Based upon the results from the analysis of dredgeability, the Silt of Reach NWK-2 has excavation characteristics classified as "good" for clamshell dredge, "good" for backhoe dredge, and "very good" for cutter dredge. Clay is classified as "poor" for clamshell dredge, "fair to poor" for backhoe dredge, and "good" for cutter dredge. Gravel is classified as "poor" for clamshell dredge, "good to poor" for backhoe dredge, and "good to fair" for cutter dredge. Most of bedrocks including Sandstone are encountered below forty feet below mean lower low water (-40 MLLW) in the most northern half of Reach NWK-2 except for northern end of Reach NWK-2; bedrocks including Sandstone and/or Shale are encountered close to or below forty feet below mean lower low water (-40 MLLW) in the northern end and southern half of Reach NWK-2. The dredgeability of each material is summarized in the below Table 14.

Matarial True	Dredge Type				
Waterial Type	Clamshell	Backhoe	Cutter	Blast	
Silt	Good	Good	Very Good	NA	
Clay	Poor	Fair to Poor	Good	NA	
Gravel	Poor	Good to Poor	Good to Fair	NA	
Bedrock (Sandstone or Shale)	Very Poor	Poor	Poor	Good	

Table 14: Dredgeability of Materials for Reach NWK-2

Consistencies of Silts were classified as very soft to soft, and the consistencies of Clays were classified as hard to medium-stiff. Consistencies of Gravels were classified as very dense to medium and RQDs of the bedrocks were estimated in the range of 26% to 100%. Based upon previous analyses, the Recent Silt sediments can be dredged by all dredges. In view of dredging records, contamination, disposal, and availability of dredging equipment, a clamshell dredge will be suitable to dredge the Recent Silt sediments in the Reach NWK-2. Cutter dredges will be suitable to dredge Clay and Gravel sediments.

As the most of sub-crops of bedrocks encountered in northern half of the Reach NWK-2 except for the northern end are located below fifty feet below mean lower low water (-50 MLLW), a heavy cutter dredge will be economical and suitable to remove the limited extent of the bedrocks to reach the proposed deepening depths. However, as the most of sub-crops of bedrocks encountered in northern end and southern half of the Reach NWK-2 are located close to or below forty feet below mean lower low water (-40 MLLW), the significant amounts of bedrocks encountered in northern end and southern half of the Reach NWK-2 will be in need of blasting to reach the proposed deepening depths. The full extent of the bedrocks in need of blasting in the northern end and southern half should need to be further investigated to estimate more accurate costs and schedules.

4.5.3. Slope Stability

Soil test boring locations PA2-479 and B4-84 were used to estimate soil strata and soil properties for analyses of slope stability as overall, the borings can provide whole soil stratification which is useful to understand stabilities of slope proposed for four feet and five feet deepenings. The soil properties of soil strata of the borings are shown in Table 15.

Boring No.	Material Type	Top Elevation of Stratum (ft)	Representative N-Value (blows/ft) /RQD (%)	Unit Weight (pcf)	Cohesion (psf)	Friction Angle (Deg.)
	Silt	-2.5	0	90	50	0
	Clay	-12.0	11	120	1370	0
	Clay	-30.0	41	130	5125	0
PA2-479	Decomposed Shale	-40.5	180	140	0	37
	Shale and Sandstone	-61.0	>30%	155	5000	40
	Silt	-6.1	0	90	50	0
B4-84	Sand	-24.5	20	120	0	33.5
	Gravel & Sand	-27.5	75	135	0	41
	Sandstone	-43.0	26%	155	5000	40

Table 15: Soil Properties of Soil Strata at Reach NWK-2

Using the above soil properties, circular and non-circular potential failure surfaces were analyzed by optimized grid-radius, entry-exit, and block analyses. As a result of simulations, the area of boring PA2-479 in the middle and northern half of the Reach NWK-2 resulted in the same lowest safety factor of slope analyses for four feet and five feet deepenings as about 1.94. The area of boring B4-84 in the southern portion of the Reach NWK-2 resulted in the same lowest safety factor of slope analyses for four feet and five feet deepenings as about 1.00. Overall, the southern portion of the Reach NWK-2 looks like not satisfying the slope stability criteria with the minimum factor of safety of 1.5, while the middle and northern half of the Reach NWK-2 meet the slope stability criteria. The summary of slope stability analyses at Reach NWK-2 is shown in Table 16.

Table 16: Summary of Slope Analyses at Reach NWK-2

Boring No.	Factors of Safety	of 4' Deepening	Factors of Safety of 5' Deepening		
	Min	Max	Min	Max	
PA2-479	1.944	2.099	1.944	1.948	
B4-84	1.002	1.102	1.002	1.039	

It is anticipated that critical slope failures in the southern portion of Reach NWK-2 including

boring B4-84 will occur as mode of local slope failure due to thick, very soft consistency top layer of silt materials underlain by relatively medium to very dense consistency layers of sand and gravel. On the other hand, it is anticipated that slope failures in the northern and middle portions of Reach NWK-2 including boring PA2-479 would not occur due to a relatively thinner, soft consistency top layer of silt materials underlain by relatively denser clay layers.

The full extent of the very soft consistency silt should need to be further investigated to determine a proper type and the scope of application of mitigation measure to stabilize the slopes in the Reach NWK-2.

Attachment #1 Figures



Figure 1: Project Areas for New York and New Jersey Harbor Deepening Channel Improvements

New York and New Jersey Harbor Deepening Channel Improvements Feasibility Study Appendix B2: Geotechnical



Figure 2: Top Contour Map of Pleistocene of Project Areas



Figure 3: Boring Location Plan of Reach AN-1



Figure 4: Boring Location Plan of Reach PJ-1

New York and New Jersey Harbor Deepening Channel Improvements Feasibility Study Appendix B2: Geotechnical



Figure 5: Boring Location Plan of Reach KVK-1



Figure 6: Boring Location Plan of Reach NWK-1



Figure 7: Boring Location Plan of Reach NWK-2

New York and New Jersey Harbor Deepening Channel Improvements Feasibility Study Appendix B2: Geotechnical



Figure 8: Cross-Section (A-A') of Reach AN-1



Figure 9: Cross-Section (A-A') of Reach PJ-1



Figure 10: Cross-Section (A-A') of Reach KVK-1


Figure 11: Cross-Section (A-A') of Reach NWK-1



Figure 12: Cross-Section (A-A') of Reach NWK-2

Attachment #2 SPT Boring Logs

 \square 98-7:1 \square SAMPLE NUMBER REMARKS IDrilling time, water loss, depth of weathering, etc., if significanti ELEVATION OEPTH CLASSIFICATION OF MATERIALS LEGEND (Description) -50 Shell Т Black CLAY, with small shell 1 Fragments 0-TD fragments (OH) 2 3 4 5 6 7 -55 8 T ġ -59.2 10 60 Dark gray CLAY, with small 11 shell fragments (OH) 12 13 14 15 65 16 TD = -66.0'Т 17 18 19 20 21 22 23 24 25 26 27 28 29 30

A١	\mathbf{VC}		98	3-58 [18	30980
-5	5.	9.		. 96	39943
LEVATION	DEPTH	SAMPLE SAMPLE	LEGEND	CLASSIFICATION OF MATERIALS (Drhill) (Description) web	REMARKS og ilma, water ioss, depth of ittering, etc., til significant)
-60 - - -65 -	1 2 3 4 5 6 7 8 9			Black CLAY, with shell fragments (OL) -60.7 Black CLAY (OL) -65.7 Black CLAY (OH)	
-70 -	11 12 13 14 15 16 17				
	18 19 20 21 22 23 24 25 26 27 28				
	29	ningeringen einerstellen understellen einerstellen understellen einerstellen understellen einerstellen			











ΡJ	-4		an a	612139	
-32	2.0	ľ		667760	
ELEVATION D	PIN KI	LECEND	CLASSIFICATION OF WATERIALS (Description)	R(WARKS 10-lating along policy loss, deput of westerring, or, W styne (corr)	
			Black to gray, organic Silty CLAY, trace Sand	tr. Shells	
35 -					
	5				
40 +	7.				
		<u>//</u>	Brown, medium to course		
			SAND, trace Silt, trace Gravel		
			Brown, fine to medium SAND, Little Grovel,		
50 - I			troce Silt		
- + 2	20 133 21 1		•	-	
- + 2	23 1				
		•••	Brown, fine to medium		
60 + 2		•••	SAND, frace Gravel, frace Silt		
3		•••			
65 + 3		•••			
· + 3 · - 3					
- 44		•••	Brown, medium to coarse SAND, little Gravel, trace Silt		
· 4 ·-75 + 4	2	• •			
- + 4 - + 4	4 77 5 1 8		Brown, medium to fine SAND, little Silt, trace		
- 4		•••	Gravel		
			Brown, medium to fine		
- 5		• •	SAND, little Gravel, trace Silt		
-85 + 5	3 🛱		BOULDERS and Decomposed		
+ 5 + 5		的肋	MICO SCHIST		-
-90 + 5		科斯			
		翻翻			
+ 6					
+ 61 + 6					
-100 + 6 + 6					
+ 70					
T G	4 1 1		. 1	1	



2	8		Nederlinesse	2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 -		666847
-1	0.	, () '			613058
ELEVATION	С ЕРТН	BLOWS	338WNN 3 Idanes	LEGENO	CLASSFICATION OF WATERIALS	REWARKS IDriving line, water loss, depth of weathering, etc., if significanti
	- 1 - 2	808 808 808 808	S1		Black, SILT (OL)	\$‡ar†:7/18/89
	- 3		·\$2 ·			Finish: 7/18/89
15-	- 5	808 808 808	\$3			-16.0'
	- 7 -	AOR AOR AOR	S4		Sand (ML)	
20-	- 9 - 10	808 808 808 808 808	S5	••••	Gray to brown, fine SAND (SP)	-19.0'
	- 11 - 12	#0H #0H #0H	S6			
-25	- 14 - 15		S7	••••		
	- 16 - 17	8	\$8	•••		
	- 18 - 19		S9 -	••••		
30-	- 20 - 21	10 8 8	S10	•••		
	- 22		\$11 \$12	••••		
-35	- 24 - 25 - 26		512	•••		
	- 27	9 13 12 18	\$13	•••		
40-	- 29 - 30	16 8 9 10		••••	Yellow modium to coarse	-30.0'
	- 31 · - 32 ·	11 10 13 18	S14	•••	SAND (SP)	
-45	- 33 - 34 - 35	5 7 7 7	\$15	•••		
	- 36 · - 37 ·	9 7 14	515	•••		-37.0'
	- 38 - 39 -	3 9 15 13	S16	• • •	SAND and coarse GRAVEL	,
50+	- 40 · - 41 ·	- 16 33 42 42		• • •		
	- 42 - - 43 -	27 25 55 98	\$17			
55-	· 45 · · 46 ·				Red-brown, fine SAND,	-44.5'
	· 47 · 48 ·	5 7 12 15	S18			
-60	· 49 · · 50 ·	20 22 22 22 22				
ĒĪ	י סוי 52 - 53 -	15 16 19 22	S19	: I		
-65-	· 54 - · 55 -	20 25 25		1		
┠╹┤	56 -			··		TD= -66.0'



	\sqrt{C}		98	3-55	665816
-6	1.	0.			615769
ELEVATION	DEPTH	SAMPLE NUMBER	LEGEND	CLASSIFICATION OF MATERIALS (Description)	REMARKS (Driving time, water loss, depth of weathering, etc., if significant)
	1 2 3 4 5			Black, very soft, slightly plastic SILT, trace shell fragments (OL) -65.0 Dark gray, very soft, slightly plastic SILT (OL)	Trace Shell Fragments 0-4'
 - 70 - 70 -	7 8 9 10			-72.0	
75 - 75 -	12 13 14 15 16			Gray, very soft, slightly plastic SILT (OL) TD = -76.0	
	17 18 19 20 21 22 23				
	24 25 26 27 28 29 30				
	31 32 33 34 35 36 37				
- - -	38 39 40 41 42 43				0
	44 45 46 47 48 49				

.

	\underline{VC}	\ / .	98	3-80	666180
-3	9.	3			615112
ELEVATION	0EPTH	SAMPLE SAMPLE	LEGEND	CLASSIFICATION OF MATERIALS (Description)	REMARKS (Drliiling lime, water loss, depth of weathering, etc., if significant)
40 -	1			Dark gray Clayey SILT, very soft, medium plasticity,	Shell fragments (0'-12')
45 - 45 - 	- 4 - 5 - 6 - 7 - 8			Gray to dark gray SILT, very soft, low to medium	-43.3 Strong Odor (8'-12') -47 3
50 - 50 -	9 10 11 12 13	WOR WOR S6 WOR S7		fragments. (OL) Dark gray Clayey SILT, very soft, medium plasticity,	-51.3
55 - 	14 15 16 17 18	WOR WOR S9		strong odor, trace shell fragments. (OL) Dark gray SILT, very soft, slightly plastic. (OL)	-53.3
60	19 20 21 22 23	WOR WOR S11		Gray to dark gray SILT, very soft, low to medium plasticity, trace fine sand, (OL)	-59.3 Shell Fragments (20'-32')
65 -	24 25 26 27	WOR S13 WOR S14		Gray Silty CLAY, very soft, highly plastic, trace shell fragments, (OH)	
70 -	28 29 30 31 32	WOR WOR S15 WOR S16		Gray Clayey SlLT, very soft, medium plasticity, trace shell fragments, (OL)	-67.3
75 -	33 34 35 36 37	WOR WOR 		Gray to light gray SILT, very soft, low to medium plasticity, trace fine sand, trace peat. (OL)	TD = -75.3'
	38 39 40 41 42 43 44 45 46 47 48			DEPTH USCS % G % S % F LL PL P1 % H2O SG 0-2 CL/OL 25.374.7 101.4	

		VC	98	3-81	666473
	-2	1.0			614535
	ELEVATION	0€РТН <u>3.0#5</u> 6- 4	LEGEND	CLASSIFICATION OF WATERIALS IDescription	REMARKS IDrilling line, water kass, depth of weathering, etc., if significanti
				Dark gray SILT, very soft, slightly plastic, trace shell fragments and fine Gravel (OL) -27 0	Trace shell fragments 0-6'
		6 WOR 7 4 9 4 10 WOR 11 4		Gray SILT, very soft, - slightly plastic. (OL) Gray SILT, very soft, slightly plastic trace fine Sand, trace shall	-29.0 Trace shell fragments 8'-12'
	35	13 4 WOR 14 WOR 15 4 16 WOR		Gray Silty CLAY, very soft, medium to high plasticity.	-33.0
	40	17 18 19 20 108 21 21		(OH) Light gray Silty CLAY, very soft, medium to high plasticity trace shell fragments, trace	Trace shell fragments 16'-42'
	45	22 wor 23 wor 24 wor 25 wor 26 wor		fine Gravel (OH)	
	-50-	27 28 WOR 29 V 30 WOR 31 V			
	-55 -	32 WOR 33 V 34 WOR 35 V 36 WOR 37		Trace wood fragments below 32.0' and some wood fragments below 40'	
	-60 -	38 x08 39 y 40 x08 41 y		Color change to gray -63.0	
	-65 -	42 WOR 43 55 44 28 45 63 46 6		Gray to light gray fine Silty SAND, very dense. (SM)	-64.0 Lorge (1/2") wood fragments 44-46'
	-70 -	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		Light gray fine SAND, dense, poorly graded. (SP)	
Ē	-75-	52 39 53 1 54 54	••••		TD = -75.0'

.

-

.



$\begin{array}{c c c c c c c c c c c c c c c c c c c $	K	Ν-	-11	}		661754
DEPTH Res Eacy of the second	-4	1 6	7 1 Iw∝			609328
1 1 Black SILT & Sand. (ML-SM) -42.7' 2 3* Red-brown grovelly. dense -47.6' 5 4* -4 -4 5 4* -4 -4 5 4* -4 -4 6 4* -4 -4 5 4* -4 -4 6 4* -4 -4 6 4* -4 -4 6 4* -4 -4 6 3* -4 -4 10 3* -4 -4 10 3* -4 -4 11 4* -4 -4 13 -4 -4 -4 13 -4 -4 -4 14 -4 -4 -4 15 -4 -4 -4 16 -4 -4 -4 17 -4 -4 -4 18 -4 -4 -4 29 -4 -4 -4	ELEVATION	DEPTH	AUMPIC	LEGEND	CLASSIFICATION OF MATERIALS (Description)	(Drilling time, woler loss, depth of weathering, etc., if significant)
-45.0-3 2 2* 5 4 2* 6 4* 7 3* 9 3* -50.0-9 3* 10 3* 11 4* 12 11 14 15 16 17 18 19 20 21 22 23 24 25 26 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 44 45 46 47 48 48 48 48 48 48 48 48 48 48		1	-3 -4 -3 -3*	┊╵╵╵ ╪┿╸┿	Black SILT & Sand. (ML-SM) Red-brown gravelly, dense glacial TILL, (GM)	-42.7'
0 13* -50.0 9 9 3* 10 3* 11 1* 12 1* 13 1* 14 15 16 17 17 18 19 20 21 22 23 24 25 26 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 41 42 43 44 44 45 46 47 48	-45.0- 	3 4 5	2* 2* 4*			-47.6'
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	-50.0-	7 · 8 ·	4* 3* 3*		Decomposed SERPENTINE Dedrock.	
13 13 14 15 16 17 18 19 19 20 21 22 22 23 23 24 25 26 27 28 29 30 31 31 32 33 34 34 35 36 39 40 41 42 43 44 45 46 46 46		10 11 12	3* 4*		TD = 53.7'	
16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 48		13 14 15				
19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48	-	16 17 18				
22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48		19 20 21				
25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48		22 ⁻ 23 ⁻ 24 ⁻				
28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48	3 • •	25 ⁻ 26 ⁻ 27 ⁻				
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		28 29 30				
34 35 36 37 38 39 40 41 42 43 44 45 46 47 48		32 33				
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		35 36 37				
$ \begin{array}{c} 40 \\ 41 \\ 42 \\ 43 \\ 44 \\ 45 \\ 46 \\ 47 \\ 48 \\ 40 \end{array} $		38 39 40				
43 44 45 46 47 48		40 41 42				
	ŀ	45 44 45				
	-	46 47 48				

.



K	VK		37	en an	661423
-4() . 1			······································	610028
ELEVATION	ДЕРТН	SAMPLE SAMPLE NUMBER	LEGEND	CLASSIFICATION OF MATERIALS (Description)	REMARKS (Drilling line, water loss, depth of weathering, etc., if significant)
	1 2 3 4 5 7 8			Brown, fine SAND, some Silt and Clay trace to some Gravel (SM)	
50.0- 50.0-	9 10 11 12 13 14 15				TD = 50.2'
	16 17 18 19 20 21				
	22 23 24 25 26 27				
	28 29 30 31 32 33				
	34 35 36 37 38 39				
	40 41 42 43 44				-
	45 46 47 48 49				

ş.

.

. -

\$









ANC 98-32 659929 613509 54.0 REMARKS (Delling line, weler loss, deeth of weathering, etc., if significant) BLOWS 6- WINNOUL LEGENO DEPTH CLASSIFICATION OF MATERIALS ELEVATION (Description) WOR No Recovery -55 1 S1 -56.0 Z Dark gray, loose to DD DD Trace Shell **S2**5333 3 moderately dense, fine Fragments(2'-20') 4 s3^{DDDDD} WOR 5 6 SAND, some Silt, trace shell 0000 -60 -60.2 fragments (SM) **S**4 7 Gray to dark gray, loose, fine 8 WOR ٥D **S5** 9 SAND, trace shell fragments)))))))10 (SP) -65 **S6** 11 1 12 13 14 WOF **S8** 15 -70 16 **S**9 17 1 -72.0 18 WOR Gray, very soft, highly 19 plastic, Silty CLAY, trace WORS11 20 -75 21 shell fragments (OL) TD = -75.0'22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49



661171	589686	REMARKS)))
		END CLASSIFICATION OF MATERIALS	III Gray SILT (ML) Red-brown Clayey SILT. little to some Gravel. trace Sand (ML)	
4B-9	-34.6	ELEVATION DEPTH BLOWS CHECE	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	

	4	<u>B</u> -	- 1		7			662279	
	-39	9.	7					590191	
	ELEVATION	ОЕРТН	<u>8.0#5</u> 6 ⁻	SAMPL	LEGEND		CLASSIFICATION OF MATERIALS (Description)	IDrilling time, water loss, depth of weathering, etc., If significant	
	40 - 	1 2 3 4					Red-brown CLAY, some Silt, little to some Gravel, trace of fine-medium Sand (SM)		-
	45 -	5	20			┦	Red-brown, fine-medium SAND,	45.0	
		7	13		İ I I I	$\left\ \right\ $	some Silt, little to some		
		8	20 40 39		↓↓ ↓↓	ł	Gravel (SM)		
	50 -	9	79		╷╷	ł	TD = -50		
÷		11							•
		12							
		14	\blacksquare	ŀ					· · ·
	55 -	15		;					
		17							
		18 19	\square						
		20	\square	•					
		21							, ,
		23	\square						1 1 a
		24	目					İ	
		26				ĺ			
		27	E	1					
		28							· .
		30							
		31 32	日						
ļ		33	E						н 1
ļ		34 35	E						
		36		-					
		37 -		:					
		39	\square						
Į		40 41	目	1					
	ł	42		1			· · ·		
		43	\blacksquare						• .
		44 45		;					
		46	E						
		.47 .48	F	1					÷
l		49		:					•••

662375 485 590294 ()) Re-19-1 199 Ç CARATER & WARDALS 142774 LEGASI Black, organic Silty CLAY R.K.S.I. -15.0' Щsэ Red-brown, Silty CLAY, little $\frac{1}{2}$ Gravel (CL) ġ 8**7**--50.0' 10 \$1 55 Red-brown, fine SAND. 11ttle 11 Silt. Ittle Grovel ISP> 12 13 14 ÷. 3 S4 42 15 Red-brown, time SAND and Gravel 捐 Glacial till (SP) 0. *\$*.0 R 4 22 22 73 -63,8' Fractured DIABASE 10 24 11 2£ Cl= -63.5'ta -58.5' Cut 3.0' Rec 86% 11 ΞŰ 1+ Pleces ROD 10% 10 218 28 11 ų, <u>99</u> C2= -68.5'+0 -73.3' 12 38 Cut 5.0' Res 58% 11 a ti 2+ Placas RGD 14% 37 10 33 15 C3- -13.5 to -78.5 8.đ 12 Cut 5.0' Rec 92% 38 11 5+ Places BCD 85% 38. 10 32 38 11 T0=-78.5" + drill rate in minutes per foot ŤĤ ** 44 12 4.3 ê s **\$**8 85 ž. 建筑 ÷.

PA 2-492	663258	
-35.51	590716	
REALEY TRANS & MARKER CONSTRAINTS & MARKER CONSTRAINTS & MARKER CONSTRAINTS & MARKER CONSTRAINTS & MARKER CONSTR	ي (در در در در در در در در در در در در در	
International state of the second state of		
Red-brown, Silty CLAY, some Gravel (CL)		
		4 4 4 4
	-50.0'	2
17 17 17 17 17 17 17 17 17 17 17 17 17 1	ver	
Si dy fid reit broot. Shale 33 - 1 -60-34 - 1		2 2 2 2 2 2 2 2 2 2 2 4 4 4 4
27 56 Red-brown, decomposed Roc 11111e fine to coarse Sam	k	4 4 4 4 4 4 4 7 4 7 7 7 7 7 7 7 7 7 7 7
	-67.0'	
-70- 16 16 17 16 16 16 16 16 16 16 16 16 16	C2= +57.0°to72.0° Cut 5-0° Rec 50% 3+ Pleces RUO 25%	
37 7 38 7 43 6 41 8	03— -/2.0'ta -/9.0' Cut 1.0' Boc 75% 3+ Pleces 800 44%	
	Ma -79.0'to -35.0'	
	Cirt 5-9" Rec 835 54 Places RCC 613	
	6535.6'to -90.0' Gut 5.0' Res 703 44 Fleces RCD 25%	
$ -90 = \begin{bmatrix} 2 & 1 & \frac{1}{2} \\ -90 & \frac{2}{56} \\ \frac{2}{56} \\ \frac{2}{56} \end{bmatrix} $	06~ -90.0'to -100' Cut 10.0' Rec 100%	
	12+ Places pcp 15%	



<u>_</u>___ 666132 Cobbles and Boulders 59203 REMARKS -67 റ П -49 м С Г Ω CLAY (CL/ML pup CLASSIFICATION OF MATERIALS HO) SILT. SAND. L (GM) (Description) H S IL Red-brown Silty B I ack Red-brown GRAVEL TIL LEGEND / 100255 S WOR S1A WOR S1A WOR S1B WOR S1B WOR S2 WOR S2 42 S8 S3B 10 12 S5 AJAMA2 AJAMALE -<u>S</u>0 S3A <u>s</u> BLOWS NOR 42 ۲ DEPTH 0 <u>0</u> 0 - NM ≤ n o r \bigcirc (\vee) - NMJUON00 ELEVATION ភេ ភ្ន -50 -02 --60



SI		31	-05-14	669100
-4	7.	· 7. ′		593305
ELEVATION	оерти	9 SAMPLE NUMBER	LEGEND CLASSIFICATION OF MATERIALS (Description)	REMARKS
50 -	1	WORS1A WORS1B WORS1B WOR	Red-brown Silty CLAY (CH/CL)	-49.2'
	5 5 6 7 8 9	WOR WOR 5 5 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	Red-brown Clayey SILT. GRAVEL, and rounded Rock fragments TILL (GM/GC)	-53.6
60 -	10 112 123 14 15	12 16 23 24 24 21 22 48 31 45 35 88 88 88 88 88 88 88 88 88 88 88 88 88		TOR = -63.3'
65 -	16 17 18 19 20 21	4 3 2 2 3	SANDSTONE	Cut 5.3'.Rec 3.8' ROD 72% FPF 4+.3.4.2.3 LCS .49.1.65
	22 23 24		· · · ·	TD = -69.5'
	25 26 27 28			
	29 30 31 32 33 34 35 36			






	P	À.	2	>	-486 662291
	-1	6.	51	99935388 	590468
an and the second second second second second second second second second second second second second second se	31X (437; 5	tpairett.	₩ Å	14521-01	(Lvifisating grading) sources
No. 1		· 1	111 ST		Black, organic Silty CLAY
10.00 miles	- 1	2 2 2			
Anos Poet- a Co	[· · ·			
and the first form		い)) 【合:	AIN 55		
		14 14	N)63 54		
	<u> </u>				
-			<u>k</u> m	الالالال بر معرفه مراجع بر مرجع محمد	Red-brown, Silty CLAY, Little -35.04
	[]	212 - 21 22			ur (1941) (CL)
	[-0]	23 74			
advite a factor		20 SV SV SV		کم تشکیر مرکز محمد محمد محمد بر محمد محمد	
Country of Country of Country of Country of Country of Country of Country of Country of Country of Country of Co	<u>_</u> 15 _	27 · 27 ·		م من من من من من من من من من من من من من	
-		· 구남 · - 강강 · · - 강경 · ·		a a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a sub a Sub a sub	-48.07
				× • •	Rectorown: SAND+ 11111 a to some Sompto saved S114. trace Grovel (SP)
		34 1	<u> </u>		Red-brown: Slity CLAY: some Gravel LOCO
	5 1	(분박 - 음위 - 동위			
and solution	59 - 	141 - 144 - 345 -	3 512	• • •	Add-brows, fine SAND, 111110 -55.0 Recomposed Shale Decomposed Shale
2. Sector occurrence		4] 42 -			58.5
Contraction (Section	60 -	4.1	s ₹	ЩY	C1= -58.5 ¹ to -62.5 ¹ or m or m or m to or m or m or m or m or
to a constraint of the constra		93 - 45 -	\$ 1		27
Summer and	63	、 点子 	*		DIABASE Cut 10.0' Rec 1000 11+ Pieces RdD 893
of the formation		- 114 - 51			•
		\$2 53	5		
		주지 555			
protection in the last		NNN 1911 1924	*	- X855 - 5 314 - 1 42 - 2367	03= -72.5'to -77.5' Cut 3.0' Rec 875
and the second second second second second second second second second second second second second second second		- 49 - 49 - 49 - 49			€Рівсин ЮОО 781.
Settemportunite		в 1977	-		C4= _*7,5_to _#2,5
in Stevene variet		63.' 51	<u>-</u> - -		Cut 5.0' Rec 355 74 Piezes ROD 683
Statutions Setti	r 1	55 . 55 . 25 .	8		
Special King	╘╼┉┥	77 상원 6.년	ċΜ		CS= -\$2.5' f0 -90.0' Cart 7.5' Ann. 1003 Lange The Sec.
		10 10 11	5		аст стиена 1920 50%
100 Metalogate		23) 17			TD= -90, 0'
1000		: 1: : 1: :::::::::::::::::::::::::::::		u ozna	+ trill rate (n mirutes per foot)



	Ρ	A 2	-45	50	672032	
	-3	7.5			214838	
	ELEVATION			CLASSIFICATION OF WATERIALS (Description)	REMARKS (Driving ilms, woler loss, depth of weolnering, etc., if significant)	
		- ! =		Black to red-brown silty CLAY,		Чалана.
	40					
	45					· .
•						-
	50 -					
		- 14 +				
		- 16 +			· · · ·	н
		- 18 +				
		- 20 - 21				
	60 -	- 22 +				
		- 24 +				·
		26				
		28				
					· · · ·	
	70					
		- 34 -			· · · ·	
		- 36 37			•	
		- 38 -			· · · · · · · · · · · · · · · · · · ·	
		- 40 =				
	— -во -	- 42 -				
		- 44 -				- - -
		46				
	85 -	- 48 - 49 - 49 - 49 - 49 - 49 - 49 - 49		Red SHALE	5.0	÷ .
		- 50				
	 90	52		· · · · · · · · · · · · · · · · · · ·		
						· ·
	95 -					
		<u>د د</u>				

Ē













Ν	BS	• •	C	8	-31	662530
	2.	4			n na na na na na na na na na na na na na	588974
ELEVATION	0EPTH	BLOWS 6"	SAMPLE	LEGEND	CLASSIFICATION OF MATERIALS IDescription	REMARKS IDritiling Time, water loss, depth of weathering, etc., if significanti
-		NOI NOI NOI NOI	S-I		Block, very soft SILT (ML)	No Recovery
5 -			5-2		Gray, fine to medium SAND,	Troce Shell fragments
	- 5 -	ł	5-3		Shell fragments (SC).	
10 -	- 7 -		5-4		Gray, medium to coarse SAND,	-9.40 Sand-sized
	- 9 -			22.22 22.22	(predominantly shell	materials mostly Shell
	- 11 -		·	נג נג נכ נכי		Tragments
15 - -	- 13 -		S-5	//	Reddish-brown, fine to	~15.9
	- 15 - - 16 -				size quartz pebbles, trace	
20 -	- 17 - - 18 -			H	Red-brown CLAY, soft,	-19.4 Firm to stiff
- · · -	- 19 -	Ľ,	S-6		plastic, some Silt. (CH)	111111 00 50111
	- 21 -	Ę	•		Red-brown CLAY, some Silt ¬ trace gravel-sized rock //	-24.4
25 -	- 23 - - 24 -	68	S-7		fragments. (CH)	Firm
	- 25 - - 26 -				Red brown CLAY, firm, highly plastic, trace to little Silt	
30 -	- 27 - - 28 -	4	S-8		trace gravel (CH).	
	- 29 - - 30 -	20				
	- 31 - - 32 -	22				-34.4
	- 33 - - 34 -	44 46	S-9		stiff to hard, medium	Stiff to hard
	- 35 - - 36 -				plasticity, trace Sand and Gravel, (CH-MH)	
40 -	- 37 - - 38 -	26 104	S-10			
	- 39 - - 40 -		\ge	0.0	COBBLES and BOULDERS,	-41.1
 45 -	- 41 - - 42 - - 47		$\backslash/$			
	- 44 - - 46 -		X		COBBLES of GRANITE. SHALE	
	- 46 -		$/ \setminus$	0.0	ana (hfki	COBBLES & BOULDERS
50 -	- 48 -			<i>a</i> <i>j</i> .		C2=-39.8to-48.0 COBBLES & BOULDERS
	- 50 - - 51 -		X	0.0.	TAN 61	-53.2
55 -	- 52 - - 53 -		/		IAN, FINE-GRAINED SANUSIONE	C3=-48.010-55.8 ROD=75%
	- 54 - - 55 -					••€€ [−] ₩₩₩
	- 56 - - 57 -					







ι SANPLE NUMBER REMARKS (Dritting time, water loss, depth of weathering, etc., if significant) DEPTH CLASSIFICATION OF MATERIALS ELEVATION LEGEND (Description) <u>R8</u> -38.4 Rock fragments and GRAVEL (GP) RB = Roller Bit 40 -Weathered Sandstone Bedrock -40.0 Light green SANDSTONE -45 -TD = -45.0





.





Attachment #3 Slope Analyses

Designed by:	JK	Date:	01 Oct., 2020
Checked by:		Date:	

















Project: NY/NJ HDCI

Reach No.: KVK-1

Reference Boring: ANC98-30

Deepening Base Elevation: EL-54.0 feet

Slope Analysis: Spencer's Method







Slope Analysis



Slope Analysis












Slope Analysis

Project: NY/NJ HDCI

Reach No.: KVK-1

Reference Boring: ANC98-31

Deepening Base Elevation: EL-55.0 feet

Materials

Organic Silt (EL-42.0ft to -44.0ft)
Sand (EL-44.0ft to -50.0ft)
Silt (below EL-50.0ft)














































































































