Westchester County Streams,
Byram River Basin
Flood Risk Management
Fairfield County, Connecticut and
Westchester County, New York

Draft Integrated Feasibility Report & Environmental Impact Statement



Appendix B4 – Structural Engineering

This appendix evaluates the alternatives associated with the removal and replacement of the Route 1 bridges over the Byram River. The purpose in exploring these alternatives is to lower the water surface elevations upstream of the existing structures which may result in a reduction of the nonstructural plan. This appendix discusses the existing conditions, proposed bridge alternatives, and impacts to the surrounding areas.

1.0 Existing Conditions

The Route 1 North Bridge (Southbound) and Route 1 South Bridge (Northbound) are classified as Urban Principal Arterial roadways (ConnDOT, 2014). The Route 1 South Bridge (Northbound) currently acts as a dam during the 2- and 1-percent storms, which inflates the water surface elevations at the North Bridge (Southbound). Per Table 7 'Flood Elevations (Existing Conditions) – Selected Area of Interest Cross Sections' from "Appendix B2 – Hydraulics" dated October 22, 2013, the Peak Water Surface Elevations at the Route 1 North Bridge (Southbound) during the 2-percent and 1-percent storm events are 16.1 ft and 17.8 ft, respectively. **Figure 1** shows the water surface profiles in the vicinity of the Route 1 Bridges during the 100- through 0.2-percent storm events as determined in Appendix B.

The existing roadway elevation at the Route 1 North Bridge is approximately 15.3ft. Therefore, the team determined it would be prudent to use the water surface elevations at the Route 1 Bridges using a HEC RAS model with the two bridges removed, but assuming the abutments and channel walls will remain. **Table 1** includes this modified existing condition as design criteria for the proposed bridge types. Refer to **Figure 2** in Section 5.0 for plan of existing (and proposed) conditions.

Table 1 Modified Existing Conditions (With Route 1 Arch Bridges Removed)

| Bridge Location (Traffic Direction) | Face of Abutment to Face of Abutment Length (ft) | Water Surface Elevation 50-yr Storm ¹ (ft) | Water Surface Elevation 100-yr Storm ¹ (ft) | Approx Roadway Elevation (Center Line) ² (ft) |
|--|---|--|---|---|
| Route 1 North (Southbound) | 80 | 12.2 | 13.2 | 15.3 |
| Route 1 South (Northbound) | 91 | 11.4 | 12.5 | 13.5 |

^{1 -} Water surface elevations are based on HEC RAS model run with the existing Route 1 arch bridges removed.

^{2 -} Existing surface information is based on available GIS.

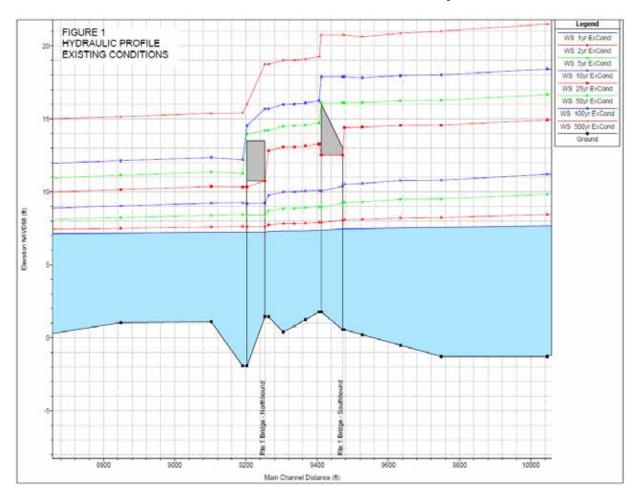


Figure 1: Water Surface Profiles during the 100-Through 0.2-percent Storm Events

2.0 Proposed Bridge Types

The team evaluated five superstructure types for the replacement structures. The superstructure types were based upon replacement structures with a 90 ft single span and elevated profile using the guidelines of the NYSDOT Bridge Manual. The following two superstructure types were identified as feasible:

1) Option 1 - Adjacent Prestressed Concrete Box Beam

This bridge type will have a total depth of 4'-6" including a 3'-3" beam, 6" deck and approximately 9" cross slope.

The advantages of this bridge type include the ability to accommodate critical vertical clearance requirements, ease of construction over a waterway and the reduction of trapped debris during high flow events.

2) Option 2 - Steel Plate Multi-girder

This bridge type will have a total depth of approximately 5'-0" including a 3'-2" girder, 9.5" deck, 3" haunch and 8" cross slope.

The advantages of this bridge type include a lighter superstructure and the ability to accommodate a large number of utilities.

The team further evaluated reducing the superstructure depth through the use of a single pier with an assumed width of 4ft. This configuration reduces the bridge to two 45ft simple spans, similar to existing conditions (See Section 3.2 for additional information). As a result of the single center pier, a third bridge type is proposed:

3) Option 3 - Prestressed Concrete Slab Unit

This type of superstructure is only proposed in conjunction with a pier and the use of two spans of 45ft. This bridge type will have a total depth of approximately 3'-0" including a 1'-9" slab unit, 6" deck and 9" cross slope.

The advantages of this bridge type include the ability to accommodate critical vertical clearance requirements, ease of construction over a waterway and the reduction of trapped debris during high flow events.

3.0 Alternatives Evaluation

Four bridge replacement alternatives were considered for the bridge types noted in Section 2.0:

3.1 Alternative 1 – Maintain Existing Roadway Profile

Alternative 1 consists of replacing both bridges with either of the two proposed feasible bridge types (Options 1 and 2) noted in Section 2.0, in their existing locations and with no roadway profile change. The impacts to surrounding properties would be minor, consisting primarily of temporary easements for grading and construction activities. ADA evaluation is outside the scope of this memorandum but should be evaluated in more depth during the design process. Although

bridge type Option 3 reduces the water surface elevation, it was not considered as part of this alternative due to the added construction cost, permitting requirements, and potential for debris collection on the pier.

3.2 Alternative 2 – Raised Roadway Profile

Alternative 2 consists of replacing both bridges with the feasible bridge types (Options 1, 2 and 3) noted in Section 2.0 in their existing locations and raising the roadway profile. **Table 2** highlights the change in the roadway profile based on adding the proposed bridge type depth (per Section 2.0) to the water surface elevation for the 100-yr storm as run through a HEC RAS model without the Route 1 bridges impeding flow, and assuming no pier (Options 1 and 2) as well as with a 4ft wide pier (Option 3).

Table 2 Alternative 2 Roadway Raised Profile Difference

| Bridge Location (Traffic Direction) | Water Surface Elevatio n 100-yr Storm (ft) | Bridge Type Option # | Bridge Depth (Type) (ft) | Approx Proposed Roadway Elevation (Center Line) (ft) | Approx Existing Roadway Elevation (Center Line) ² (ft) | Roadway Profile Difference from Existing Profile (ft) |
|---|---|-------------------------------|---|--|--|--|
| Route 1 North | 13.2 ¹ | 1 | 4.5 (Box Beam) | 17.7 | 15.3 | +2.4 |
| (Southbound) | 13.2 | 2 | 5.0 (Steel Girder) | 18.2 | 15.5 | +2.9 |
| Route 1 South | 12.5 ¹ | 1 | 4.5 (Box Beam) | 17.0 | 13.5 | +3.5 |
| (Northbound) | 12.5 | 2 | 5.0 (Steel Girder) | 17.5 | 13.3 | +4.0 |
| Route 1 North (Southbound) | 13.5 ³ | 3 | 3.0 (2 Span - Prestressed Slab Unit) | 16.5 | 15.3 | +1.2 |
| Route 1 South (Northbound) | 12.6 ³ | 3 | 3.0 (2 Span - Prestressed Slab Unit) | 15.6 | 13.5 | +2.1 |

^{1 -} Water surface elevations are based on HEC RAS model run with the existing Route 1 arch bridges removed.

3.3 Alternative 3 – Single Bridge North (Southbound) Location

Alternative 3 consists of replacing both bridges with a new single bridge using either of the two feasible proposed bridge types (Options 1 and 2) in Section 2.0, with a single span, and a raised roadway profile in the North Bridge (Southbound) location. As a result of removing the South Bridge (Northbound), the bridge for this alternative will have an approximate width of 70ft comprised of two sidewalks (8ft each), two shoulders (3ft each), and four travel lanes (12ft each). Bridge type Option 3 was not considered as part of this alternative because the impacts are a result of the increased Right of Way (ROW) width required to accommodate the traffic from the South Bridge as opposed to the difference in elevation of the bridge and roadway.

^{2 -} Existing surface information is based on available GIS.

^{3 -} Water surface elevation is based on HEC RAS model run with 4ft wide pier at the Route 1 bridges.

Table 3 Alternative 3 Single Bridge North (Southbound) Location

| Bridge Location (Traffic Direction) | Water Surface Elevation 100-yr Storm ¹ (ft) | Bridge Depth (Type) (ft) | Approx Proposed Roadway Elevation (Center Line) (ft) | Approx Existing Roadway Elevation (Center Line) ² (ft) | Roadway Profile Difference from Existing Profile (ft) |
|---|---|--------------------------------|--|---|--|
| Route 1 North (Southbound) | 13.2 | 4.5 (Box Beam) | 17.7 | 15.3 | +2.4 |
| | 13.2 | 5.0 (Steel Girder) | 18.2 | 15.5 | +2.9 |

^{1 -} Water surface elevations are based on HEC RAS model run with the existing Route 1 arch bridges removed.

3.4 Alternative 4 – Single Bridge South (Northbound) Location

Alternative 4 consists of replacing both bridges with a new single bridge using either of the two feasible proposed bridge types (Options 1 and 2) in Section 2.0, with a single span, and a raised roadway profile in the South Bridge (Northbound) location. As a result of removing the North Bridge (Southbound), the bridge for this alternative will have an approximate width of 70ft comprised of two sidewalks (8ft each), two shoulders (3ft each), and four travel lanes (12ft each). Bridge type Option 3 was not considered as part of this alternative because the impacts are a result of the increased ROW width required to accommodate the traffic from the North Bridge as opposed to the difference in elevation of the bridge and roadway.

Table 4 Alternative 4 Single Bridge South (Northbound) Location

| Bridge Location (Traffic Direction) | Water Surface Elevation 100-yr Storm ¹ (ft) | Bridge Depth (Type) (ft) | Approx Proposed Roadway Elevation (Center Line) (ft) | Approx Existing Roadway Elevation (Center Line) ² (ft) | Roadway Profile Difference from Existing Profile (ft) |
|---|---|--------------------------------|--|---|--|
| Route 1 South (Northbound) | 12.5 | 4.5 (Box Beam) | 17.0 | 13.5 | +3.5 |
| | 12.5 | 5.0 (Steel Girder) | 17.5 | 13.5 | +4.0 |

 $¹⁻Water \ surface \ elevations \ are \ based \ on \ HEC \ RAS \ model \ run \ with \ the \ existing \ Route \ 1 \ arch \ bridges \ removed.$

3.5 Impacts of Alternatives

Table 6 includes impacts to the surrounding properties based on each of the four alternatives for bridge type Options 1 and 2. These impacts are approximate and based on Right of Way information from available GIS and will require verification through a complete ground survey. **Table 6** does not include impact to utilities (private or public) and drainage which will also require a full survey beyond GIS data to determine types, location, inverts, etc. It also does not explore potential archaeological impacts as it pertains to historical significance/preservation. In addition to **Table 6**, **Table 5** contains the approximate limits of roadway work necessary to achieve the profile difference of the bridge types from Section 2.0 for Alternatives 1-4. The lengths were measured from the limits of each bridge location (i.e. outside the span lengths).

^{2 -} Existing surface information is based on available GIS.

^{2 -} Existing surface information is based on available GIS.

Bridge type Option 3 (2 span - Prestressed Concrete Slab Unit) was proposed to determine if a reduction in the superstructure depth would minimize impacts to the properties surrounding the bridges. Introducing a pier reduces the depth of the superstructure (as compared to bridge type Option 1 and 2) by 1.5-2.0ft, but increases the water surface elevation to 13.5ft (0.3ft increase from the no pier options). This results in an approximate proposed roadway elevation of the North Bridge of 16.5ft, which is 1.2-1.7ft lower than using Option 1 and 2. However, when compared to the existing roadway profile, this still results in a 1.2ft increase in roadway elevation. In order to accommodate this change, impacts to the surrounding properties immediately adjacent to the bridge (604 N Main St – "Dougie's Stand-By", "Carvel", "Sara's Food Mart"; 11 Hillside Ave – "Accessible Mobility, LLC"; 13 Riverdale Ave – "Clay Health Club & Spa"; 780 W Putnam Ave – "Exxon" gas station") will be similar to those noted in **Table 6** (Alternative 2). Option 3 will also require the following additional considerations:

- · Job specific permitting;
- · Longer construction duration;
- · Greater impact to streambed work in the water;
- · Additional cofferdam for pier construction;
- Temporary construction access to pier location;
- Additional impacts to traffic during construction and traffic detours that will need to be in effect for a longer duration;
- · Additional maintenance due to likely debris build up at pier;
- Overall increased cost compared to Options 1 and 2.

Table 5 Limits of Roadway Work per Alternative

| Alternative | Bridge Location (Traffic Direction) | Limits (Compass Heading) | Approx Length of Roadway To be Reconstructed (ft) |
|---|--|--------------------------|---|
| _ | Route 1 North | NE | 150 |
| 1 (Maintain Eviating | (Southbound) | SE | 150 |
| (Maintain Existing Roadway Profile) | Route 1 South | NE | 150 |
| , | (Northbound) | SW | 150 |
| | Route 1 North | NE | 250 |
| 2 | (Southbound) | SW | 200 |
| (Raised Roadway Profile) ¹ | Route 1 South (Northbound) | NE | 300 |
| , | | SW | 250 |
| 3 | | NE | 250 |
| (Single Bridge North (Southbound) Location) | Route 1 North (Southbound) | SW | 200 |
| 4 | | NE | 300 |
| (Single Bridge South (Northbound) Location) | Route 1 South (Northbound) | SW | 250 |

^{1 -} Limits of Roadway Work utilizing bridge type Option 3 (2 Span - Prestressed Concrete Slab Unit) similar to Alternative 2.

Table 6 Property Impacted by Various Alternatives

| | - | Property | | Impact Ty | у ре | |
|---|--|--|--|---|--|---|
| Alternative | Address | Type- Description | Site Access (Ingress/Egress) | Structural/Property Use | Aesthetic | Non Structural/Minor |
| 1 (Maintain Existing Roadway Profile) | 604 N Main St, Port Chester, NY 10573 | Commercial – "Dougie's Stand By" | Outdoor patio access (chain link gate) and building entrance below grade and will require a step(s) to accommodate grading (even with maintaining existing roadway profile). Permanent easement may be required. | Patio and outdoor seating (approx. 50sqft) below grade and will require a step(s) to accommodate grading (even with maintaining existing roadway profile). Permanent easement may be required. | - N/A | - N/A |
| | | | | | | |
| | 11 Hillside Ave, Port Chester, NY 10573 | Commercial – "Accessible Mobility, LLC" | · N/A | Parking lot would require regrading or 2- 3' retaining wall at back of sidewalk to accommodate grade change. Temporary easement will likely be required. | · N/A | - N/A |
| | Hillsdale Ave and Riverdale Ave, Port Chester, NY 10573 | Commercial – "West Conn" Convenient Store | Parking lot driveway on Hillside Ave will need to be reconstructed and regraded to accommodate grade change. Temporary easement will likely be required. | Parking lot would require regrading to accommodate grade change. Temporary easement will likely be required. | · N/A | · N/A |
| | 13 Riverdale Ave, Port Chester, NY 10573 | Commercial – "CLAY Health Club & Spa" | Emergency egress stairs would require relocation or a 2-3' retaining wall at back of sidewalk and drainage considerations to accommodate grade change. Permanent easement may be required. | Basement windows (8 EA) approx. 6" above the base of the building would require 2-3' retaining wall at back of sidewalk and drainage considerations due to grade change. Permanent easement may be required. | · N/A | · N/A |
| 2 (Raised Roadway Profile) ¹ | 777 W Putnam Ave, Greenwich, CT 06830 | Commercial – "Fifth Street Finance" | Driveways (approx. 150' from East Approach) will need to be reconstructed or relocated to accommodate grade change. Temporary easement will likely be required | - N/A | '777' stone sign at driveway entrance will need to be removed and reset to accommodate grade change. Temporary easement will likely be required. | Grass at back of sidewalk will need to be regraded to accommodate grade change. Temporary easement will likely be required. |
| | 780 W Putnam Ave, Greenwich, CT 06830 | Commercial – "Exxon" gas station | Driveway (approx. 50' from East limit of North Bridge) nearly at grade with roadway gutter line will require regrading or relocating to accommodate grade change. Wheelchair ramp will require reconstructing or relocating to accommodate grade change. Driveway (approx. 50' from East limit of South Bridge) at grade with roadway gutter line may require relocating to accommodate grade change. Temporary easement will likely be required. | 'Tiger Mart' Building below grade, set back approx. 40' from North Bridge and 50' from South Bridge roadway gutter line will require drainage considerations to alleviate flooding to accommodate grade change (Note: There are existing catch basins around building. May require relocating these catch basins to accommodate grade change). Temporary easement will likely be required. | Gas pumps and pavement below grade will require drainage considerations to alleviate flooding to accommodate grade change (Note: There are existing catch basins around building. May require relocating these catch basins to accommodate grade change). Temporary easement will likely be required. | - N/A |
| 2 (Raised Roadway Profile) ¹ | 604 N Main St, Port Chester, NY 10573 | Commercial – "Dougie's Stand By" | Outdoor patio access (~48" chain link gate) below grade and will require relocating and a 0.5-1' retaining wall to accommodate grade change. Building entrance set back approx. 15' from roadway gutter line is below grade | Patio and outdoor seating (approx. 50sqft) below grade and will require a 0.5-1' retaining wall to accommodate grade change. | If retaining wall is required, the patio view of the roadway will be blocked, and may not be aesthetically pleasing to patrons. | Shrubs/bushes between patio and parking lot entrance will need to be removed and new shrubs/bushes planted to accommodate grade change. |



| | | Property | | Impact Ty | ре | |
|--|------------------------|---|--|---|---|--|
| Alternative | Address | Type- Description | Site Access (Ingress/Egress) | Structural/Property Use | Aesthetic | Non Structural/Minor |
| | | | and will require drainage considerations and 0.5-1' retaining wall at back of sidewalk to accommodate grade change. Driveway (approx. 65'from West Limit of bridge) will require reconstructing or relocating to accommodate grade change. Permanent easement may be required. | Parking lot will require regrading to accommodate grade change. Permanent easement may be required for patio. Temporary easement will likely be required for parking lot. | | |
| | St, Port Chaster NV | Commercial – "Sara Food Mart" | - N/A | Parking lot (shared with "Dougie's Stand By" and "Carvel Ice Cream") will require regrading to accommodate grade change on the North Bridge. Parking lot (shared with "Allstate" and "Portchester Auto Spa") will require regrading to accommodate grade change on the South Bridge, potentially removing 2 parking spaces. Temporary easement will likely be required for parking lot. | - N/A | - N/A |
| | St, Port Chester NV | Commercial – "Carvel Ice Cream" | - N/A | Parking lot (shared with "Dougie's Stand By" and "Sara Food Mart") will require regrading to accommodate grade change. Temporary easement will likely be required. | - N/A | Landscaping in front of "Carvel Ice Cream" may need to be removed and new shrubs/bushes planted to accommodate grade change. |
| | St, Port | Commercial – "Gulf" Gas Station | Gas station entrance will likely require drainage consideration to accommodate grade change in roadway. Permanent easement will likely be required. | Driveway/entrance to gas pumps nearly at grade with existing South Bridge roadway gutter line and will need to be reconstructed or relocated to accommodate grade change. Gas pumps may need to be reset or relocated to accommodate grade change. Permanent easement will likely be required. | - N/A | - N/A |
| | St, Port Chaster NV | Commercial – "Portchester Auto Spa" | 3 garage openings are nearly at grade with existing South Bridge roadway gutter line. Raising the roadway profile will require drainage considerations to alleviate flooding or relocating of the garage openings. Permanent easement will likely be required. | Driveway/entrance to garage openings nearly at grade with existing South Bridge roadway gutter line and will need to be relocated to accommodate grade change. Parking lot will need to be regraded to accommodate grade change. Permanent easement will likely be required. | If reduction in the parking lot in front of "Portchester Auto Spa" is required, the number of cars allowed to park may also be reduced. | · N/A |
| 3 (Single Bridge North - Southbound) Location) | Chester NV | Commercial – "Accessible Mobility, LLC" | Driveway Entrance on Riverside Ave may need to be relocated to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | Parking lot would be reduced in size to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | Additional pedestrian accommodations would be required to cross Hillside Ave as a four lane roadway. | · N/A |

| | Property | | Impact Ty | ре | |
|-------------|--|--|--|---|--|
| Alternative | Type- Address Description | Site Access (Ingress/Egress) | Structural/Property Use | Aesthetic | Non Structural/Minor |
| | Hillsdale Ave and Riverdale Ave, Port Chester, NY 10573 Commercial – "West Conn" Convenient Store | Parking lot driveway on Hillside Ave will need to be relocated and regraded to accommodate grade change and two additional lanes of traffic from removing South (Northbound) Bridge. Entrance to convenient store may require relocation or step down to accommodate grade change and two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | Parking lot would be reduced in size to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | Expanded roadway may detract customers. Additional pedestrian accommodations would be required. | - N/A |
| | 13 Riverdale Commercial – Ave, Port "CLAY Health Chester, NY Club & Spa" 10573 | Emergency egress stairs would require relocation to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | Building footprint would be within sidewalk/shoulder due to two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | - N/A | · N/A |
| | 777 W Putnam Commercial – Ave, "Fifth Street Greenwich, CT 06830 | Driveways (approx. 150' from East limit of North Bridge) will need to be reconstructed or relocated to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | - N/A | '777' stone sign at driveway entrance will need to be relocated to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | Grass at back of sidewalk will need to be regraded to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. |
| | 780 W Putnam Commercial – Ave, "Exxon" gas Greenwich, CT 06830 | Driveway (approx. 50' from East Approach) nearly at grade with roadway gutter line will require relocating to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Wheelchair ramp will require relocating to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | 'Tiger Mart' Building below grade, set back approx. 40' from North Bridge roadway gutter line will require drainage considerations to alleviate flooding to accommodate grade change and two additional lanes of traffic from removing South (Northbound) Bridge. 2 parking spaces will be removed to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | Gas pumps and pavement below grade will require drainage considerations to alleviate flooding to accommodate grade change and two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. | "Exxon" Sign will need relocating to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Air and vacuum station will need relocating to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Trash bin (large) and surrounding fence will require relocating to accommodate two additional lanes of traffic from removing South (Northbound) Bridge. Permanent easement will likely be required. |
| | 604 N Main St, Port Chester, NY 10573 Commercial – "Dougie's Stand By" | Outdoor patio access (~48" chain link gate) below grade and within limits of expanded roadway needed to accommodate two additional lanes of traffic due to removal of South Bridge. Driveway (approx. 65' from West Limit of bridge) will require relocating to accommodate two additional lanes of traffic due to removal of South Bridge. | Patio and outdoor seating (approx. 50sqft) below grade and within limits of expanded roadway needed to accommodate two additional lanes of traffic due to removal of South Bridge. Parking lot will need to be reduced (removing 2-3 parking spots) and regraded to accommodate grade change and two additional lanes of traffic due to removal of South Bridge. | Reduced patio size may impact patron desire to sit closer to roadway or relocate outdoor seating will be required. | Shrubs/bushes between patio and parking lot entrance will need to be removed and new shrubs/bushes planted to accommodate additional lanes and grade change. |

| | | Property | Impact Type | | | | |
|--|---|--|--|---|--|---|--|
| Alternative | Address | Type- Description | Site Access (Ingress/Egress) | Structural/Property Use | Aesthetic | Non Structural/Minor | |
| | | | Permanent easement will likely be required. | Permanent easement will likely be required. | | | |
| 3 (Single Bridge North - Southbound) Location) | 604 N Main St, Port Chester, NY 10573 | Commercial – "Sara Food Mart" | - N/A | Parking lot will need to be reduced (removing 2-3 parking spots) and regraded to accommodate grade change and two additional lanes of traffic due to removal of South Bridge. Permanent easement will likely be required. | - N/A | - N/A | |
| | 604 N Main St, Port Chester, NY 10573 | Commercial – "Carvel Ice Cream" | · N/A | Parking lot will need to be reduced (removing 2-3 parking spots) and regraded to accommodate grade change and two additional lanes of traffic due to removal of South Bridge. Permanent easement will likely be required. | - N/A | Landscaping in front of "Carvel Ice Cream" may need to be removed to accommodate two additional lanes of traffic due to removal of South Bridge. Permanent easement will likely be required. | |
| | | T T | | | | | |
| 4 (Single Bridge South - Northbound) Location) | 780 W Putnam Ave, Greenwich, CT 06830 | Commercial – "Exxon" gas station | Driveway (approx. 50' from East limit of South Bridge) at grade with roadway gutter line will require relocating to accommodate grade change and two additional lanes of traffic from removing North (Southbound) Bridge. Wheelchair ramp will require relocating to accommodate grade change and two additional lanes of traffic from removing North (Southbound) Bridge. Permanent easement will likely be required. | 'Tiger Mart' Building below grade, set back approx. 50' from South Bridge roadway gutter line will require 3-4' retaining wall at the back of sidewalk and/or drainage considerations to alleviate flooding to accommodate grade change and two additional lanes of traffic from removing North (Southbound) Bridge. 2-3 parking spaces will be removed to accommodate two additional lanes of traffic from removing North (Southbound) Bridge. Permanent easement will likely be required. | Gas pumps and pavement below grade will require 3-4' retaining wall at the back of sidewalk and/or drainage considerations to alleviate flooding to accommodate grade change and two additional lanes of traffic from removing North (Southbound) Bridge. Permanent easement will likely be required. | "Exxon" Sign will need relocating to accommodate two additional lanes of traffic from removing North (Southbound) Bridge. Electrical panel will need relocating to accommodate two additional lanes of traffic from removing North (Southbound) Bridge. Permanent easement will likely be required. | |
| | 604 N Main St, Port Chester, NY 10573 | Commercial – "Sara Food Mart" | · N/A | Parking lot will need to be reduced (removing 2-3 parking spots) and regraded to accommodate grade change and two additional lanes of traffic due to removal of North Bridge. Permanent easement will likely be required. | N/A | · N/A | |
| 4 | 602 N Main St, Port Chester, NY 10573 | Commercial – "Gulf" Gas Station | Gas station entrance will likely require drainage consideration to accommodate grade change in roadway and two additional lanes of traffic due to removal of North Bridge. Permanent easement will likely be required. | Driveway/entrance to gas pumps nearly at grade with existing roadway gutter line and will need to be relocated to accommodate grade change and two additional lanes of traffic due to removal of North Bridge. Gas pumps will need to be relocated to accommodate grade change and two additional lanes of traffic due to removal of North Bridge. | - N/A | · N/A | |

| | | Property | Impact Type | | | |
|--|--|---|--|---|--|----------------------|
| Alternative | Address | Type- Description | Site Access (Ingress/Egress) | Structural/Property Use | Aesthetic | Non Structural/Minor |
| (Single Bridge South - Northbound) | | | | Permanent easement will likely be required. | | |
| Location) | 602 N Main St, Port Chester, NY 10573 | Commercial – "Portchester Auto Spa" | 3 garage openings are nearly at grade with existing South Bridge roadway gutter line. Raising the roadway profile and adding two additional lanes of traffic will require relocating of the garage openings due to removal of the North Bridge. Permanent easement will likely be required. | Driveway/entrance to garage openings nearly at grade with existing South Bridge roadway gutter line and will need to be relocated to accommodate grade change and two additional lanes of traffic due to removal of the North Bridge. Parking lot will need to be reduced (3-4 spots) to accommodate grade change and additional lanes of traffic due to removal of North Bridge. Permanent easement will likely be required. | Reduction in the parking lot in front of "Portchester Auto Spa" will be required to accommodate the two additional lanes of traffic, which will reduce the number of cars allowed to park. | · N/A |

^{1 –} Property Impact utilizing bridge type Option 3 (2 Span -Prestressed Concrete Slab Unit) similar to impacts listed for Alternative 2.

3.7 Scour Analysis

The proposed 80 feet north bridge and the proposed 90 feet south bridge were analyzed for scour utilizing HEC-18 methodology. The proposed bridges were analyzed for abutment and contraction scour for the 1-percent and 0.2-percent floods. No piers are present, so pier scour was not evaluated. The scour results below represent theoretical scour and do not account for any rock that may be present to prevent scour. The presence of rock could limit the scour drastically from the theoretical results. Borings will be required in the channel at the beginning and the end of the proposed bridge locations to verify the presence of rock and establish the limiting elevation and rock quality for resisting scour. This limitation should be evaluated for final design. Rock parameters will be needed to evaluate the rock for potential scour in accordance with HEC-18 Quarrying and Plucking methodology if competent rock is not found at a shallow depth. **Table 7** and **Table 8** below summarize the results from the HEC-18 theoretical results.

Table 7 North Bridge Scour Results

| Location | Abutment Scour (ft) | Contraction Scour (ft) | Total Scour (ft) |
|----------------|---------------------|------------------------|------------------|
| Left Abutment | 17.4 | N/A | 17.4 |
| Right Abutment | 27.0 | N/A | 27.0 |

Table 8 South Bridge Scour Results

| Location | Abutment Scour (ft) | Contraction Scour (ft) | Total Scour (ft) |
|----------------|---------------------|------------------------|------------------|
| Left Abutment | 8.6 | N/A | 8.6 |
| Right Abutment | 18.2 | N/A | 18.2 |

In accordance with FHWA guidance, countermeasures may be utilized to resist scour for existing structures. For these two bridges, there is little potential for contraction scour, given the vertical abutments and relatively similar opening size of the bridge and adjoining channel upstream and downstream. It is recommended that abutment foundations be founded on competent rock. If it is found that the bedrock does not limit the predicted scour depth, riprap countermeasure would be an appropriate protection against scour in accordance with guidance provided by FHWA HEC-23. The rock size has been calculated such that a minimum D50 of 1.3 feet would be appropriate for the estimated future conditions flow characteristics for both bridges. Refer to Section 5.0 Attachments for riprap calculations.

3.6 Recommendation

Based on Section 3.5 and **Tables 5** and **6**, it is the team's recommendation to further investigate Alternative 2 – Raised Roadway Profile using bridge type Option 1 – Adjacent Prestressed Concrete Box Beam. Alternative 1 – Maintain Existing Roadway Profile does not alleviate flooding concerns and was therefore removed from consideration. Alternatives 3 and 4 – Single Bridge North and South, respectively, would require fee property takings and likely require removal of

existing structures in addition to significant temporary and permanent traffic pattern impacts. For this reason, Alternatives 3 and 4 were not considered feasible. Lastly, additional bridge type Option 3 - Prestressed Concrete Slab Unit with Pier was proposed to reduce the superstructure depth, thereby reducing impact to the surrounding properties. It was determined that the roadway and property impacts would be similar to bridge type Options 1 and 2 despite a reduced superstructure depth and would also create additional short term and long term impacts as listed in Section 3.5. Due to these additional impacts and limited reduction of roadway and property impacts, bridge type Option 3 is not recommended.

A full topographic survey of the existing bridges and surrounding properties is recommended to help refine and confirm the impacts listed in **Table 6** for Alternative 2. Refer to Section 5.0 Attachments for plan, profiles and sections for the recommended treatment and limits of grading likely required at each property to accommodate the raised roadway profiles. Also included in the Attachments is a conceptual bridge cost estimate compiled using NYSDOT items and historical weighted average item pricing with contingency for the recommended alternative.

4.0 References

"Byram River Flood Study – Appendix B on Hydraulics" Prepared by CDM Smith for the City of Greenwich, CT on March 14, 2014

"Connecticut State Numbered Routes and Roads", Connecticut Department of Transportation Bureau of Policy and Planning Office of Roadway Information Systems Roadway Inventory Section, Pages 1-6, December 31, 2014

"A Policy on Geometric Design of Highways and Streets", American Association of State Highway and Transportation Officials (AASHTO), 6th Edition, 2011

"NYSDOT Bridge Manual", January 2008, 1st Edition, April 2014, Addendum #3

"NYSDOT Pay Item Catalog & Weighted Average Bid Pricing", 2015

5.0 Attachments



| JOB: | Byram River B | ridge (N | lorth) | SHEET | | |
|------------|---------------|----------|-----------|-------|--|--|
| SUBJECT: | | | | | | |
| CALC'D BY: | Gambill | DATE: | 26-Mar-18 | OF | | |
| CHEK'D BY | Corley | DATF. | 26-Mar-18 | 1 | | |

| | | ABU | TMENT RIPE | RAP CALCU | LATION | NS - 1 | 00-Yr | Desig | า | | | | |
|-----------------------|---------------------------|------------------|-----------------|-------------------|-----------|---------------|----------|----------|--------------|----------|-----------|---------|----------|
| | | _ | Des | ign Guidelir | ne 14 | | | | | | | | |
| Paramete r | Unit | Left Overbank | Channel | Right Overbank | | | | | | | | | |
| у | (ft) | 0.00 | 12.55 | 0.00 | Flow De | epth | | | | | | | |
| Q | (cfs) | 0 | 6428 | 0 | Dischar | ge | | | | | | | |
| Α | (sf) | 0.0 | 793.3 | 0.0 | Flow Ar | ea | | | | | | | |
| Setback | (ft) | 0 | N/A | 0 | | | | | | | | | |
| SBR | | 0.0 | N/A | 0.0 | Setback | (Ratio | : Setbac | k/Flow | Depth | | | | |
| Vavg | (fps) | | 8.10 | | If SBR< | 5 for E | oth Abu | tments | Compute Va | vg of Fu | ll Bridge | • Openi | ng) |
| Vavg | (fps) | N/A | 8.10 | N/A | If SBR> | 5 for E | ither Ab | utment | , Compute Va | avg of O | verbank |) | |
| Vavg | (fps) | N/A | 8.10 | N/A | If SBR< | 5 for E | ither Ab | utment | , Compute Ch | naracter | istic Va | /g) | |
| Vdes | (fps) | N/A | 8.1 | N/A | | | | | | + | | | |
| Fr | | #VALUE! | 0.40 | #VALUE! | | | | | | | | | |
| pill-through Abutme | ents | | | | | | | | | | | | |
| D50 | (ft) | #VALUE! | N/A | #VALUE! | If Fr>0. | 8, use | HEC-23 | Eq.14.2 |) | | | | |
| D50 | (ft) | #VALUE! | 1.26 | #VALUE! | If Fr<0. | 8, use | HEC-23 | Eq. 14. | 1 | | | | |
| | | | ons for Spillth | | | | | | | | | | |
| | | Note: Equati | ons for Vertic | al Abutment | s, K=0.69 | (Fr>0 | .8) and | 1.02 (Fr | <0.80) | | | | |
| Ise the larger of the | D ₅₀ values fo | or both abutme | nts | | | | | | | | | | l |
| | D50= | 1.26 | ft | | | | | | | | | | |
| | | | | | | | | | | | | | |
| Apron Width | (ft) | 0 | or 25 ft, fron | n toe | | | | | | | | | |
| Riprap Thickness | (ft) | 1.89 | T=1.5*D50 ar | | an D100 | | D100 = | 2.52 | | | | | |
| High Water Elev. | (ft) | 434.6 | . 1.5 255 ui | | 1 | | | 2.52 | | | | | |
| Top Elevation | (ft) | 436.6 | | | | | | | | | | | |
| . op Eteration | (10) | .55.5 | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | <u> </u> |

Reference: HEC-23 3rd Ed., Design Guideline 14, 2009



JOB: Byram River Bridge (South)

SHEET

SUBJECT:

CALC'D BY: Gambill

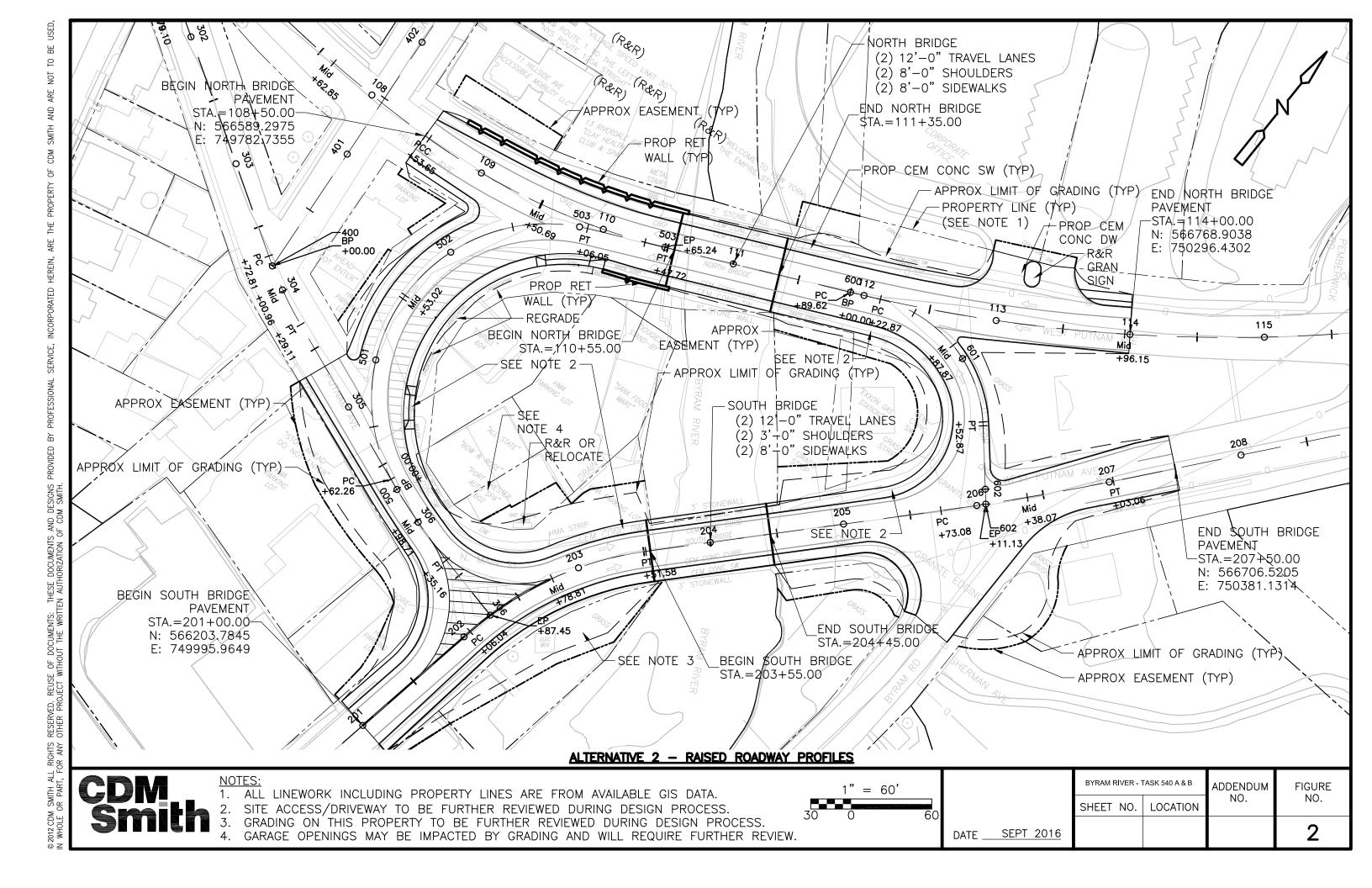
CHEK'D BY: Corley

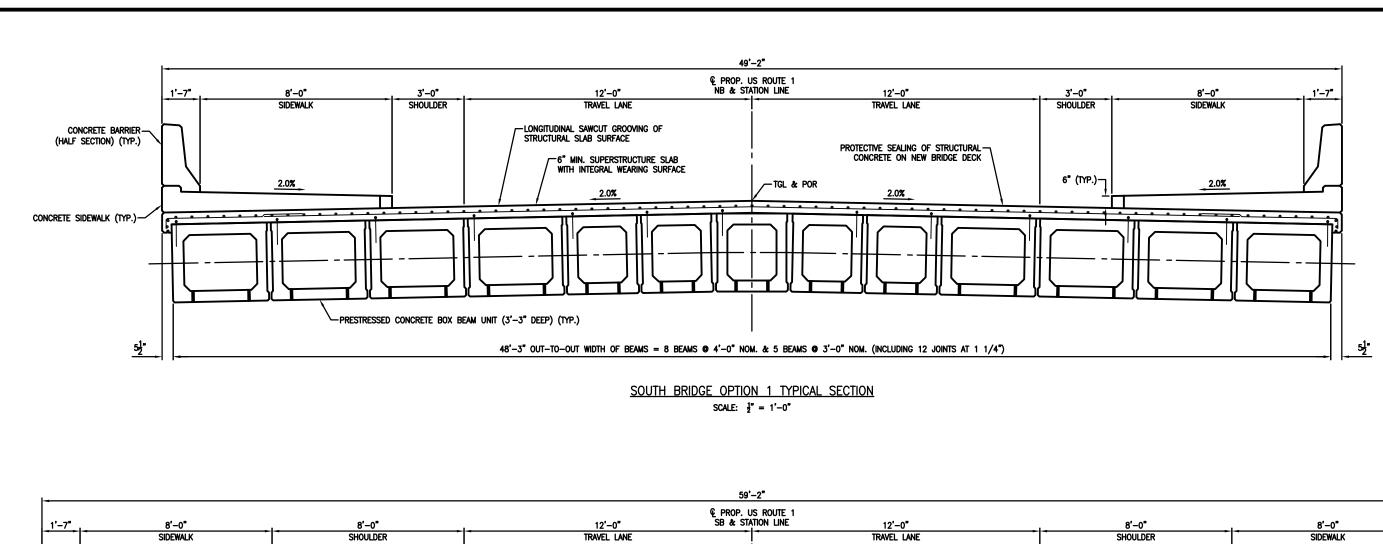
DATE: 26-Mar-18

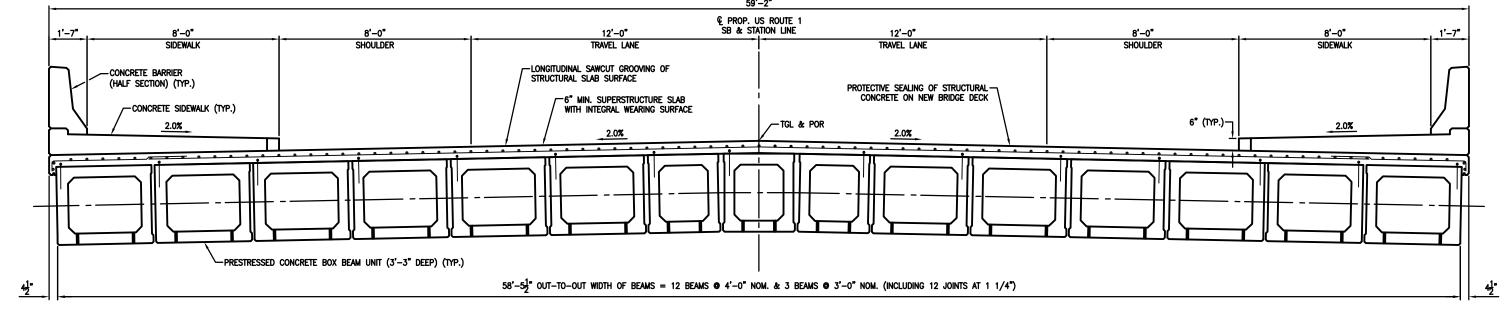
1

| D50 (ft) #VALUE! N/A #VALUE! If Fr>0.8, use HEC-23 Eq.14.2 D50 (ft) #VALUE! 1.26 #VALUE! If Fr<0.8, use HEC-23 Eq.14.1 Note: Equations for Spillthrough Abutments, K=0.61 (Fr>0.80) and 0.89 (Fr<0.80) Note: Equations for Vertical Abutments, K=0.69 (Fr>0.8) and 1.02 (Fr<0.80) Use the larger of the D ₅₀ values for both abutments D50= 1.26 ft Apron Width (ft) 0 or 25 ft, from toe Riprap Thickness (ft) 1.89 T=1.5*D50 and not less than D100 D100 = 2.52 High Water Elev. (ft) 434.6 | | | ADIIT | MENT DIDE | DAD CALC | II ATION | IC 4 | 00 V- | Docie | | | | | |
|--|---|---------------------------|------------------|----------------|----------------|-----------|--------|----------|---------|-----------|----------|-----------|-----|----------|
| y (ft) 0.00 11.77 0.00 Flow Depth Q (cfs) 0 6428 0 Discharge A (sf) 0.0 793.3 0.0 Flow Area Setback (ft) 0 N/A 0 If SBR-5 for Both Abutments, Compute Vavg of Full Bridge Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank, Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.11 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank, Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank, Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank, Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank, Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Paul Bridge Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Paul Bridge Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Paul Bridge Vavg (fps) N | | | ADU | | | | 12 - 1 | 100-11 | Desig | <u> </u> | | | | |
| y (ft) 0.00 11.77 0.00 Flow Depth Q (cfs) 0 6428 0 Discharge A (sf) 0.0 793.3 0.0 Flow Area Setback (ft) 0 N/A 0 SBR 0.0 N/A 0.0 Setback Ratio: Setback/Flow Depth Vavg (fps) 8.10 If SBR-5 for Both Abutments, Compute Vavg of Full Bridge Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank Vdes (fps) N/A 8.1 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.1 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.1 N/A If Fro.0.8, use HEC-23 Eq. 14.1 D50 (ft) #VA | Paramete r | Unit | Left Overbank | | | | | | | | | | | |
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| Setback (ft) 0 N/A 0 Setback Ratio: Setback/Flow Depth SBR 0.0 N/A 0.0 Setback Ratio: Setback/Flow Depth Vavg (fps) 8.10 If SBR-5 for Both Abutments, Compute Vavg of Full Bridge Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Vavg of Overbank, Vavg (fps) N/A 8.10 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.1 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.1 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.1 N/A If SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.1 N/A If For SBR-5 for Either Abutment, Compute Characteristic Vav Vdes (fps) N/A 8.1 N/A If For SBR-5 for Either Abutment, Compute Characteristic Vav Spill-through Abutments N/A #VALUE! If Fr-0.8, use HEC-23 Eq. 14.2 | Q | (cfs) | 0 | 6428 | 0 | Dischar | ge | | | | | | | |
| SBR | Α | (sf) | 0.0 | 793.3 | 0.0 | Flow Ar | ea | | | | | | | |
| Vavg (fps) 8.10 If SBR<5 for Both Abutments, Compute Vavg of Full Bridge Vavg (fps) N/A 8.10 N/A If SBR<5 for Either Abutment, Compute Vavg of Overbank | Setback | (ft) | 0 | N/A | 0 | | | | | | | | | |
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| Vavg (fps) N/A 8.10 N/A If SBR>5 for Either Abutment, Compute Vavg of Overbank, Vavg Vavg (fps) N/A 8.10 N/A If SBR<5 for Either Abutment, Compute Characteristic Vavance in the Spill of the Sp | SBR | | 0.0 | | 0.0 | | | | | • | | | | <u> </u> |
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| Vdes (fps) N/A 8.1 N/A Image: Note of the post of the | Vavg | (fps) | N/A | 8.10 | N/A | | | | | | | | • | |
| Fr #VALUE! 0.42 #VALUE! WALUE! Spill-through Abutments Material Spill Spi | Vavg | (fps) | N/A | 8.10 | N/A | If SBR<5 | for E | ither Ab | utment | , Compute | Characte | ristic Va | vg) | |
| Fr #VALUE! 0.42 #VALUE! WALUE! Spill-through Abutments Material Spill Spi | | | | | | | | | | | | | | |
| Spill-through Abutments D50 (ft) #VALUE! N/A #VALUE! If Fr>0.8, use HEC-23 Eq.14.2 D50 (ft) #VALUE! 1.26 #VALUE! If Fr<0.8, use HEC-23 Eq. 14.1 | | (fps) | | | | | | | | | | | | |
| D50 (ft) #VALUE! N/A #VALUE! If Fr>0.8, use HEC-23 Eq.14.2 D50 (ft) #VALUE! 1.26 #VALUE! If Fr<0.8, use HEC-23 Eq. 14.1 Note: Equations for Spillthrough Abutments, K=0.61 (Fr>0.80) and 0.89 (Fr<0.80) Note: Equations for Vertical Abutments, K=0.69 (Fr>0.8) and 1.02 (Fr<0.80) Use the larger of the D ₅₀ values for both abutments D50= 1.26 ft Apron Width (ft) 0 or 25 ft, from toe Riprap Thickness (ft) 1.89 T=1.5*D50 and not less than D100 D100 = 2.52 High Water Elev. (ft) 434.6 | • | | #VALUE! | 0.42 | #VALUE! | | | | | | | | | |
| D50 (ft) #VALUE! 1.26 #VALUE! If Fr<0.8, use HEC-23 Eq. 14.1 Note: Equations for Spillthrough Abutments, K=0.61 (Fr>0.80) and 0.89 (Fr<0.80) Note: Equations for Vertical Abutments, K=0.69 (Fr>0.8) and 1.02 (Fr<0.80) Use the larger of the D ₅₀ values for both abutments D50= 1.26 ft Apron Width (ft) 0 or 25 ft, from toe Riprap Thickness (ft) 1.89 T=1.5*D50 and not less than D100 D100 = 2.52 High Water Elev. (ft) 434.6 | | | | | | | | | | | | | | |
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| Riprap Thickness (ft) 1.89 T=1.5*D50 and not less than D100 D100 = 2.52 High Water Elev. (ft) 434.6 — </td <td></td> | | | | | | | | | | | | | | |
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| High Water Elev. (ft) 434.6 | Apron Width | (ft) | 0 | or 25 ft, fron | n toe | | | | | | | | | |
| High Water Elev. (ft) 434.6 | Riprap Thickness | (ft) | 1.89 | T=1.5*D50 ar | nd not less th | an D100 | | D100 = | 2.52 | | | | | |
| | | (ft) | 434.6 | | | | | | | | | | | |
| Top Elevation (ft) 436.6 | Top Elevation | (ft) | 436.6 | | | | | | | | | | | |
| | · · · · · · | , , | | | | | | | | | | | | |
| | | | | | · - | | | | | | | 1 | | . – |

Reference: HEC-23 3rd Ed., Design Guideline 14, 2009

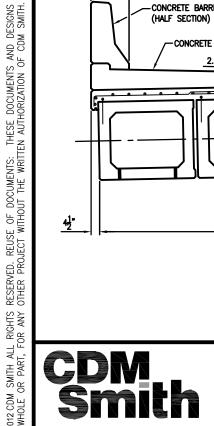






NORTH BRIDGE OPTION 1 TYPICAL SECTION

SCALE: ½" = 1'-0"



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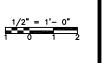
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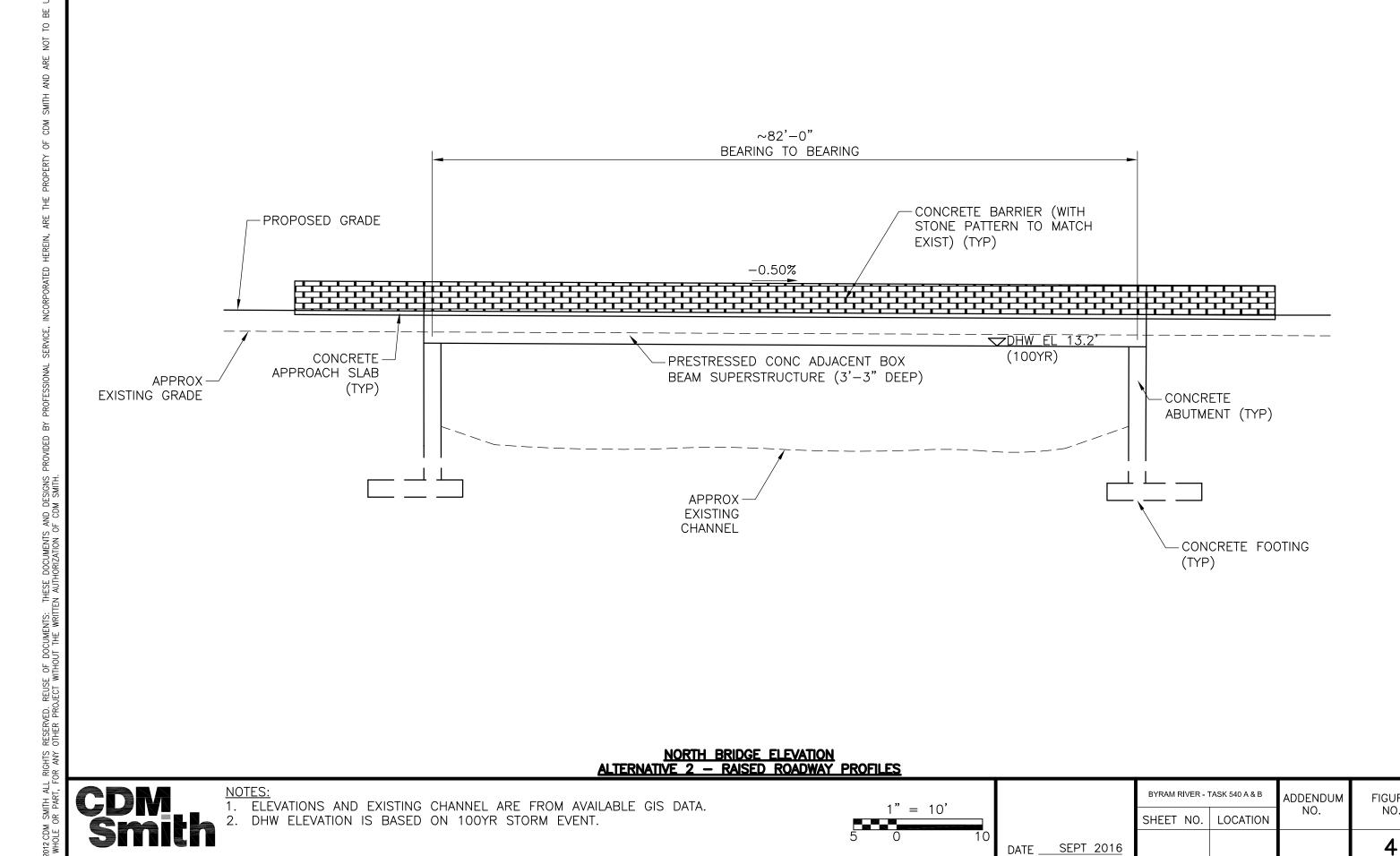
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| | | BYRAM RIVER - ⁻ | TASK 540 A & B | ADDENDU |
|------|-----------|----------------------------|----------------|---------|
| | | SHEET NO. | LOCATION | NO. |
| DATE | SEPT 2016 | | | |

FIGURE NO.



NORTH BRIDGE ELEVATION ALTERNATIVE 2 - RAISED ROADWAY PROFILES

NOTES:

1. ELEVATIONS AND EXISTING CHANNEL ARE FROM AVAILABLE GIS DATA.

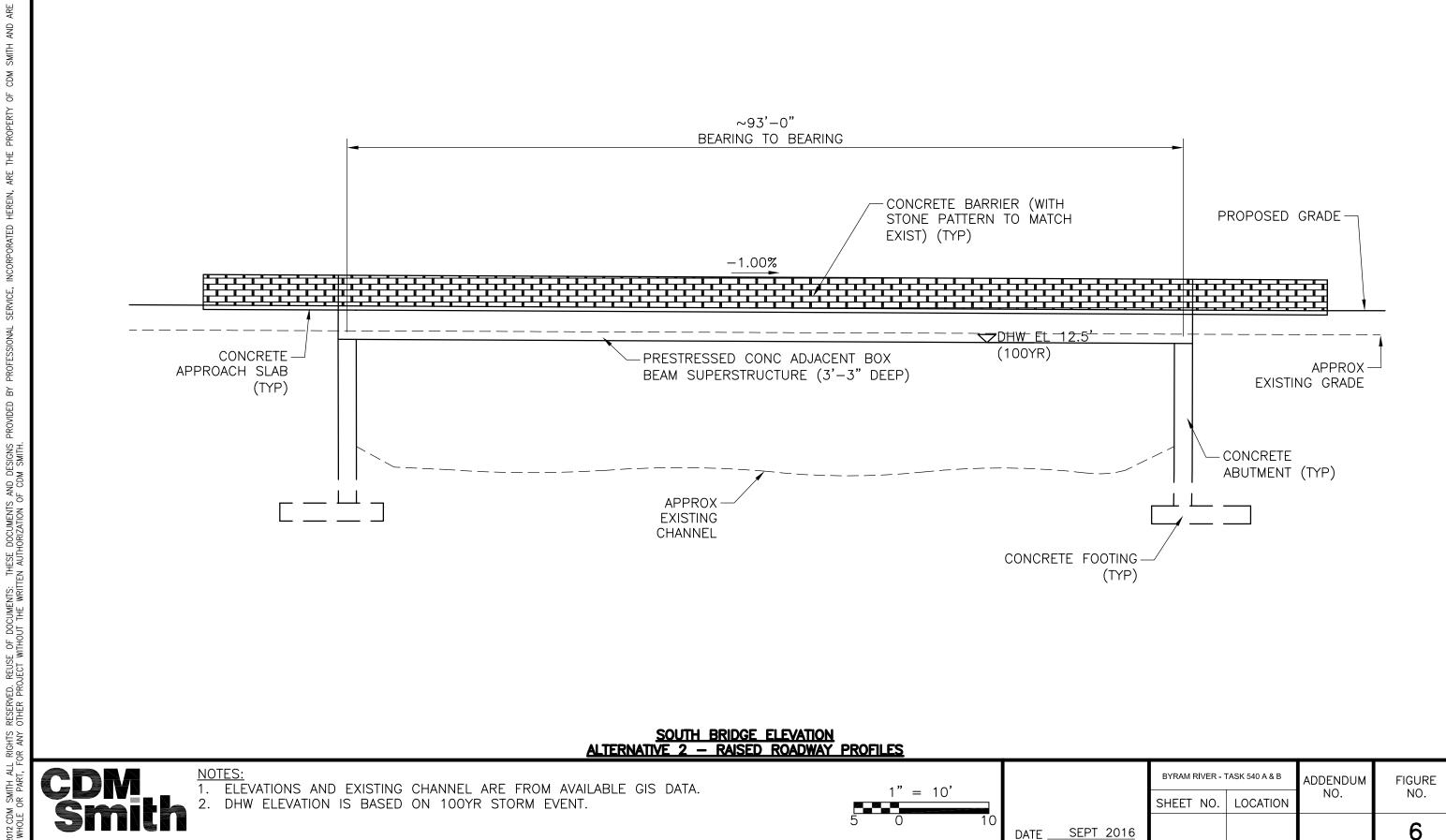
2. DHW ELEVATION IS BASED ON 100YR STORM EVENT.

| | 1" = | 10' | |
|---|------|-----|----|
| 5 | 0 | | 10 |

BYRAM RIVER - TASK 540 A & B **ADDENDUM** NO. SHEET NO. LOCATION SEPT 2016 DATE _

4

FIGURE NO.



SOUTH BRIDGE ELEVATION ALTERNATIVE 2 - RAISED ROADWAY PROFILES

NOTES:

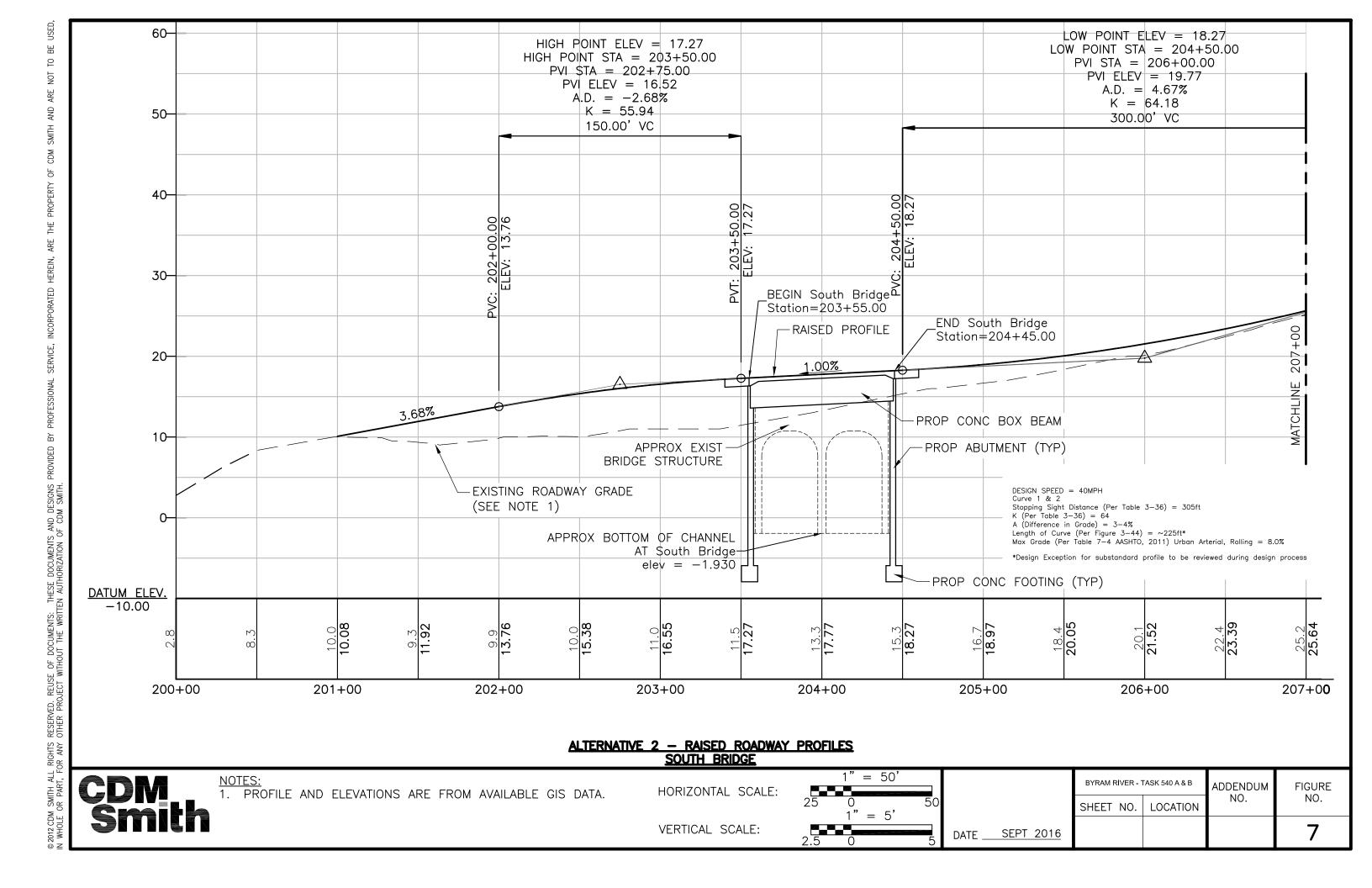
1. ELEVATIONS AND EXISTING CHANNEL ARE FROM AVAILABLE GIS DATA.

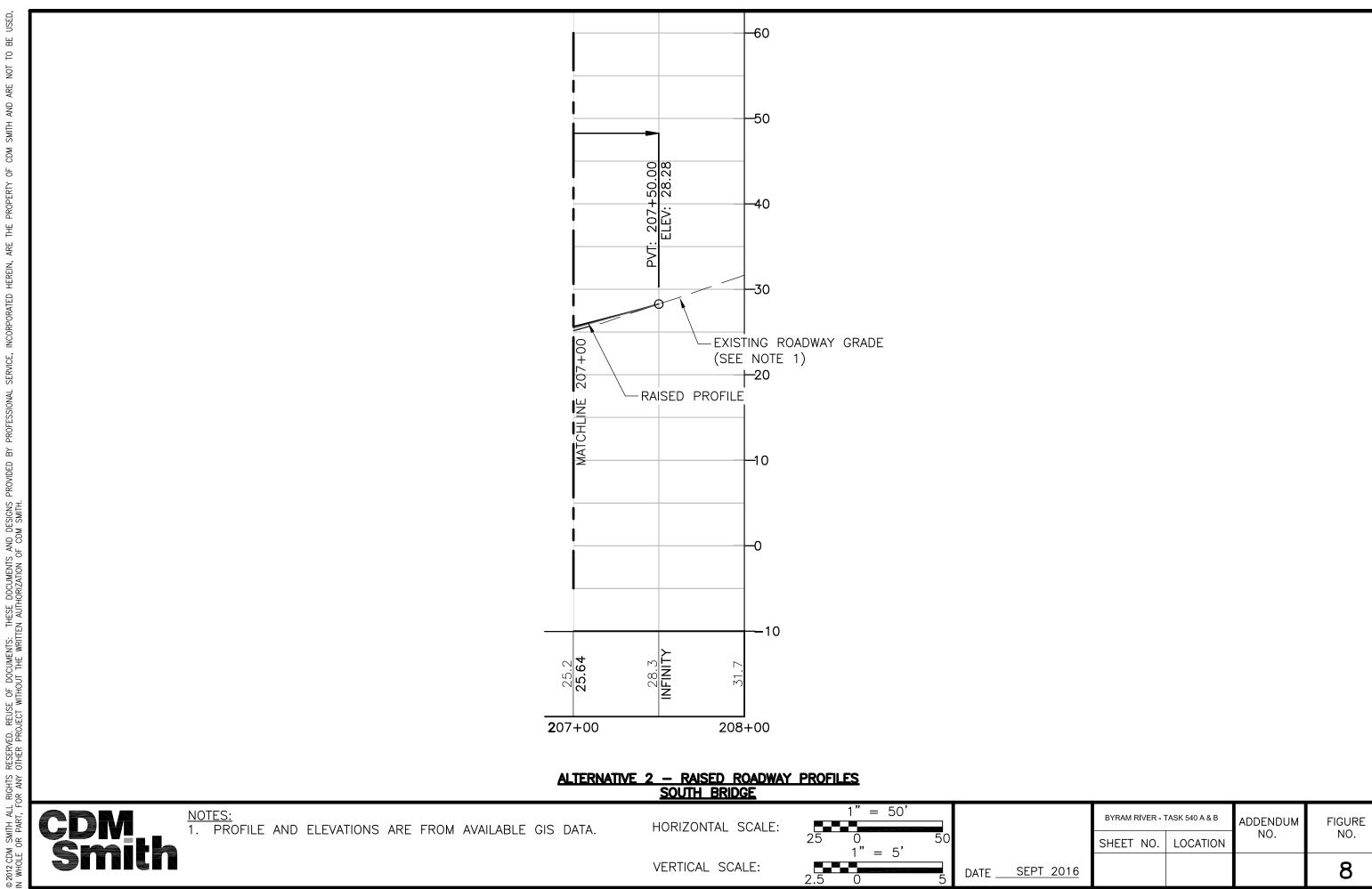
2. DHW ELEVATION IS BASED ON 100YR STORM EVENT.

| 1" | = | 10' | | |
|-----|---|-----|----|--|
| 5 0 | | | 10 | |

BYRAM RIVER - TASK 540 A & B SHEET NO. SEPT 2016 DATE _

ADDENDUM FIGURE NO. NO. LOCATION 6





VERTICAL SCALE:

DATE _

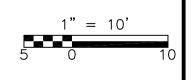
SEPT 2016

8

CDM Smith

CDM Smith

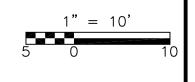
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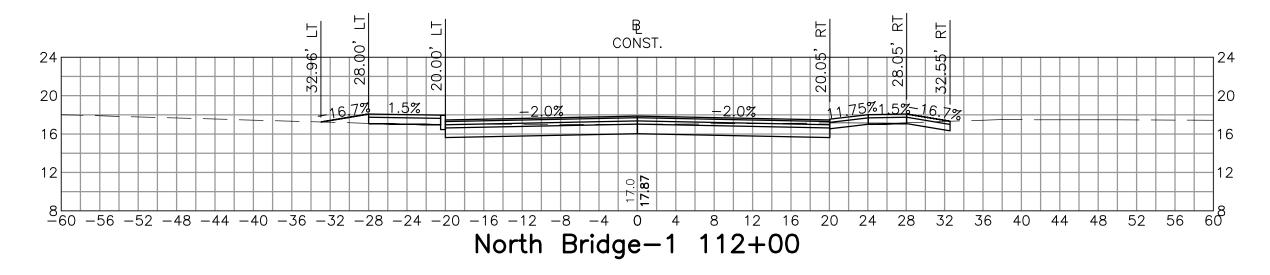
| | BYRAM RIVER - 1 | ΓASK 540 A & B | ADDENDUM | FIGURE |
|----------------|-----------------|----------------|----------|--------|
| | SHEET NO. | LOCATION | NO. | NO. |
| DATE SEPT 2016 | | | | 9 |

CDM Smith

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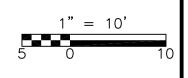


| | BYRAM RIVER - 7 | ΓASK 540 A & B | ADDENDUM | FIGURE |
|-----------------------|-----------------|----------------|----------|--------|
| | SHEET NO. | LOCATION | NO. | NO. |
| OATE <u>SEPT 2016</u> | | | | 10 |



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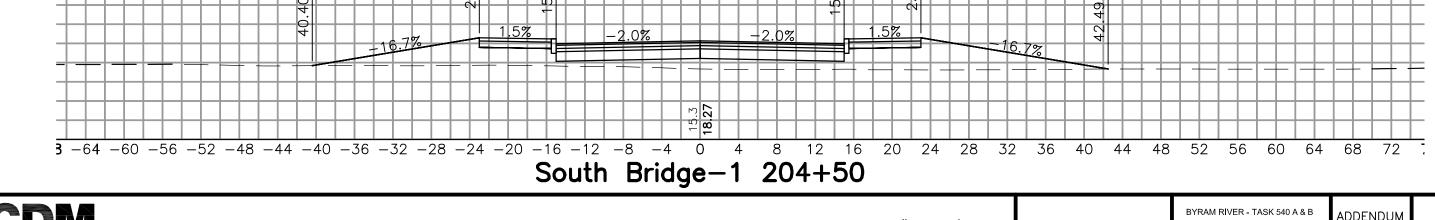


| | BYRAM RIVER - 7 | ΓASK 540 A & B | ADDENDUM | FIGURE |
|-----------------------|-----------------|----------------|----------|--------|
| | SHEET NO. | LOCATION | NO. | NO. |
| DATE <u>SEPT 2016</u> | | | | 11 |

2

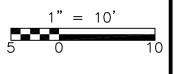
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SEPT 2016

DATE _

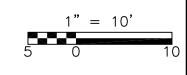
| BYRAM RIVER - TASK 540 A & B | ADDENDUM | | |
|------------------------------|----------|-----|-----|
| SHEET NO. | LOCATION | NO. | NO. |
| | | | 13 |

-16 -12 -8 -4 0 4 8 12 16
South Bridge-1 205+25

CDM Smith

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| | BYRAM RIVER - 1 | ΓASK 540 A & B | ADDENDUM | FIGURE |
|-----------------------|-----------------|----------------|----------|--------|
| | SHEET NO. | LOCATION | NO. | NO. |
| DATE <u>SEPT 2016</u> | | | | 14 |



Project:

Byram River Rte 1 Bridge Alternatives

CDM Smith Project #: Task 540 - Byram River

Date: 08/30/16

Atlernative 2 - Raised Roadway Profiles with

Bridge Type Option 1 - Prestressed Concrete Box Beams

| Item No. | Description | Units | Quantity | Unit Price | Total Cost |
|-------------|---|-------|-----------|--------------|-------------|
| 201.07 | CLEARING AND GRUBBING | ACRE | 0.25 | \$14,618 | \$3,655 |
| 202.2 | REMOVING OLD BITUMINOUS CONCRETE OVERLAY | SY | 800.00 | \$15 | \$12,064 |
| 203.02 | UNCLASSIFIED EXCAVATION AND DISPOSAL | CY | 8,300.00 | \$25 | \$209,243 |
| 203.06 | SELECT FILL | CY | 3,300.00 | \$47 | \$156,387 |
| 304.0001002 | FINE GRADING OF EXISTING SUBBASE | SF | 68,200.00 | \$1 | \$40,920 |
| 304.0197061 | CRUSHED STONE AGGREGATE SUBBASE COURSE (12" THICKNESS) | CY | 2,600.00 | \$79 | \$204,282 |
| 402.128202 | F2 TOP COURSE HMA, 80 SERIES COMPACTION (2" THICKNESS) | TON | 900.00 | \$93 | \$83,439 |
| 402.258902 | F9 BINDER HMA, 80 SERIES COMPACTION (2" THICKNESS) | TON | 900.00 | \$100 | \$89,550 |
| 402.376902 | F9 BASE HMA, 60 SERIES COMPACTION (4" THICKNESS) | TON | 1,800.00 | \$83 | \$150,048 |
| 520.0900001 | SAW CUTTING ASPHALT CONCRETE | LF | 900.00 | \$3 | \$2,259 |
| 520.5 | SAWING CONCRETE | LF | 64.00 | \$7 | \$437 |
| 608.0101 | CONCRETE SIDEWALKS AND DRIVEWAYS | CY | 500.00 | \$536 | \$267,780 |
| 688.01 | WHITE PREFORMED REFLECT PAVEMENT STRIPES | LF | 2,900.00 | \$2 | \$5,800 |
| 688.02 | YELLOW PREFORMED REFLECT PAVEMENT STRIPES | LF | 1,100.00 | \$3 | \$3,267 |
| 999.1 | UTILITY ADJUSTMENTS (INCLUDES DRAINAGE, SEWER, UTIL POLES, WATER) | EA | 34.00 | \$475 | \$16,134 |
| 999.2 | BRIDGE REPLACEMENT - NORTH - OPTION 1 - BOX BEAMS | LS | 1.00 | \$1,224,800 | \$1,224,800 |
| 999.3 | BRIDGE REPLACEMENT - SOUTH - OPTION 1 - BOX BEAMS | LS | 1.00 | \$1,478,400 | \$1,478,400 |
| 999.4 | RETAINING WALL | CY | 200 | \$1,200 | \$240,000 |
| 999.5 | LANDSCAPING | LS | 1 | \$ 10,000.00 | \$10,000 |
| 999.6 | TRAFFIC CONTROL/DETOUR SETUP | LS | 1 | \$419,846.51 | \$419,847 |
| | | | | Subtotal | \$4,618,312 |
| | INCIDENTALS (5%) | | | | \$231,000 |
| | | | | Subtotal | \$4,849,312 |
| | CONTINGENCY (25%) | | | | \$1,212,000 |
| | | | | Subtotal | \$6,061,312 |
| 697.03 | FIELD CHANGE PAYMENT (5%) | LS | 1 | \$303,000 | \$303,000 |
| | | | | Subtotal | \$6,364,312 |
| 699.040001 | MOBILIZATION (10%) | LS | 1 | \$636,000 | \$636,000 |
| | | | | Subtotal | \$7,000,312 |
| | INFLATION (5%) | | | | \$350,000 |
| | | | | TOTAL | \$7,350,312 |

NOTES:

¹⁾ NO EASEMENTS/TAKING PRICES WERE CONSIDERED AS PART OF THIS ESTIMATE

²⁾ ITEM 999.1 ASSUMES ADJUSTMENTS OF FRAMES, GRATES, ETC. AND DOES NOT INCLUDE REPLACEMENT OF ANY UTILITIES

| ESTIMATE OF QUANTITIES | | | | | |
|------------------------|---|-------|----------|--|--|
| ITEM NO. | DESCRIPTION | UNITS | QUANTITY | | |
| 201.07 | CLEARING AND GRUBBING | ACRE | 0.25 | | |
| 202.2 | REMOVING OLD BITUMINOUS CONCRETE OVERLAY | SY | 800 | | |
| 203.02 | UNCLASSIFIED EXCAVATION AND DISPOSAL | CY | 8300 | | |
| 203.06 | SELECT FILL | CY | 3300 | | |
| 304.0001002 | FINE GRADING OF EXISTING SUBBASE | SF | 68200 | | |
| 304.0197061 | CRUSHED STONE AGGREGATE SUBBASE COURSE (12" THICKNESS) | CY | 2600 | | |
| 402.128202 | F2 TOP COURSE HMA, 80 SERIES COMPACTION (2" THICKNESS) | TON | 900 | | |
| 402.258902 | F9 BINDER HMA, 80 SERIES COMPACTION (2" THICKNESS) | TON | 900 | | |
| 402.376902 | F9 BASE HMA, 60 SERIES COMPACTION (4" THICKNESS) | TON | 1800 | | |
| 520.0900001 | SAW CUTTING ASPHALT CONCRETE | LF | 900 | | |
| 520.5 | SAWING CONCRETE | LF | 64 | | |
| 608.0101 | CONCRETE SIDEWALKS AND DRIVEWAYS | CY | 500 | | |
| 688.01 | WHITE PREFORMED REFLECT PAVEMENT STRIPES | LF | 2900 | | |
| 688.02 | YELLOW PREFORMED REFLECT PAVEMENT STRIPES | LF | 1100 | | |
| 999.1 | UTILITY ADJUSTMENTS (INCLUDES DRAINAGE, SEWER, UTIL POLES, WATER) | EA | 34 | | |
| 999.2 | BRIDGE REPLACEMENT - NORTH - OPTION 1 - BOX BEAMS | LS | 1 | | |
| 999.3 | BRIDGE REPLACEMENT - SOUTH - OPTION 1 - BOX BEAMS | LS | 1 | | |
| 999.4 | RETAINING WALL | CY | 200 | | |
| 999.5 | LANDSCAPING | LS | 1 | | |
| 999.6 | TRAFFIC CONTROL/DETOUR SETUP | LS | 1 | | |
| 697.03 | FIELD CHANGE PAYMENT (5%) | LS | 1 | | |
| 699.040001 | MOBILIZATION (10%) | LS | 1 | | |

CDM Smith

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Item: 201.07 | Clea | ring and Grubb | ing | Units: ACRE |
|---|---------------------------|-----------------|-----|-------------|
| escription: r removal of brush to accommodate grading. 1 ACRE | E = 43,560 SQFT. Assume a | pprox 0.25 ACRE | | |
| lculations: | | | | |
| 0.25 ACRE = 10890 SQFT | | | | |
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| - | | | 1 | |
| | Item Total: 0.25 | ACRE | | |
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CDM Smith

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Item: 202.2 | Removing Old bituminous | concrete overlay | Units: SY |
|---|-----------------------------------|--------------------------|--------------------------------------|
| Description: Removal of Misc. HMA. Assume 10% of all HMA F | Roadway surface. | | |
| Calculations: Total Roadway Area (Inside curb line) | - Total Internal Turnaround Area | = Total Bit Conc Roadway | / 9 SQFT/SQYD |
| 136085 SQFT | - 67930 SQFT | = 68155 SQFT | / 9 = 7573 SQYD * 0.10 = 757 SQYD |
| | | | |
| | | | |
| | Item Total: 757.3 SY Say: 800 SY | | |

CDM Smith

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Project: | | Byram River Rte 1 Bridge Alternatives | | | |
|----------------|-------|---------------------------------------|----|---|--|
| Sheet No: | 3 | of | 22 | _ | |
| Calculated By: | | Date: | | _ | |
| Checked By: | | Date: | | _ | |
| Total: | 8,300 | CY | | | |

| Item: 203.02 | | Unclassified Excavation and Disposal | Units: CY | | | | |
|---|-----------------|---|---------------|--|--|--|--|
| Description: Assume all excavation associated with roadway, sidewalk, driveway and grading beyond back of walk. Assume 2ft depth of excavation. | | | | | | | |
| Calculations: | | | | | | | |
| Sidewalk, Driveway Width (ft) | * 2* | Project Length (according to alignment lengths and assuming 2 sidewalks) (ft) | | | | | |
| 8 | * 2 * | Hillside Ave Turnaround West Putnam Ave (550 + 150 + 200 | e + | | | | |
| | | West Putnam Ave | SQFT | | | | |
| Beyond back of sidewalk (5ft) | * 2* | Project Length (according to alignment lengths and assuming 2 sidewalks) (ft) Hillside Ave Turnaround West Putnam Ave | | | | | |
| 5 | * 2 * | (550 + 150 + 200 | + | | | | |
| | | West Putnam Ave Turnaround East + 650 + 125) = 16750 | SQFT | | | | |
| Total Roadway Area (Inside curb line |) - Total Inter | rnal Turnaround Area = Total Bit Conc Roadway | | | | | |
| 136085 SQFT | - | 67930 SQFT = 68155 | SQFT | | | | |
| | | Subtotal 111705 | SQFT | | | | |
| | | Subtotal (CF) 223410 | CF / 27 CF/CY | | | | |
| | | Conversion (CY) 8274.44 | CY | | | | |
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| | Item Total: | 8,274.4 CY | | | | | |
| | Say: | 8,300 CY | | | | | |
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75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Project: | | Byram River Rte 1 Bridge Alternatives | | | | | | |
|----------------|-------|---------------------------------------|----|---------|--|--|--|--|
| Sheet No: | 4 | of | 22 | | | | | |
| Calculated By: | | Date: | | <u></u> | | | | |
| Checked By: | | Date: | | <u></u> | | | | |
| Total: | 3.300 | CY | | | | | | |

| Item: 203.06 | | | Select | t Fill | Units: CY |
|---|--------------|--------------------------------------|-----------|--|---------------|
| escription: Il needed to raise the North Bridg | ge (Hillside | e Ave) and South Bridge (West Putnar | m Ave) ៧ | oadway profiles. | |
| alculations: | | | | | |
| Per AutoCAD Civil 3D Mas | ss/Haul Di | agram which uses Average End Area | to calcul | late Cut/Fill. Below is the Net volume (Fi | II) required. |
| North Bridge Fill Volume (CY) | + | South Bridge Fill Volume (CY) | = | Total Volume of Fill (CY) | |
| 115 | + | 3130 | = | 3245 CY | |
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| | | Item Total: 3,245.0 | CY | | |
| | | | CY | | |

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Item: 304.0001002 | Fine Grading of existing subbase | Units: SF |
|---|---|-----------|
| Description: Area is per roadway limits. | | |
| Calculations: | | |
| Total Roadway Area (Inside curb line) | - Total Internal Turnaround Area = Total Bit Conc Roadway | |
| 136085 SQFT | - 67930 SQFT = 68155 SQFT | |
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PHONE: (617) 452-6000

| Project: | Byram River Rte 1 Bridge Alternatives | | | | | | |
|----------------|---------------------------------------|----|-------|----|--|--|--|
| Sheet No: | 6 | of | 2 | 22 | | | |
| Calculated By: | | | Date: | | | | |
| Checked By: | | | Date: | | | | |
| Total: | 2.600 | CY | | | | | |

| Item: 304.019 | 7061 | | | Crushed | Stone aggre | egate subbase | course (12 | " Thickne | ss) | | Units: C | CY |
|--|------------------|----------------|-----------------|-------------|-------------|---------------|------------|-----------|---------------|------|----------|----|
| escription: rea of subbase is per r | padway limits. A | ssumed depth o | f crushed stone | e is 12". | | | | | | | | |
| alculations: | | | | | | | | | | | | |
| otal Roadway Area (Ins | ide curb line) | - Total | Internal Turnar | round Area | = | Total Roadway | * 1 (ft) | / : | 27 CF/CY | | | |
| 136085 | SQFT | - | 67930 | SQFT | = (| 68155 SQFT | * 1 |) / : | 27 = 2524.259 | O CY | | |
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| | | | | Item Total: | 2,524.3 | CY | | | | | | |
| | | | | Say: | 2,600 | CY | | | | | | |

75 State St, Suite 701
Boston, MA 02109
PHONE: (617) 452-6000

| Item: 402.128202 | F2 Top Course HMA, 80 Series Compaction (2" Thickness) | Units: TON |
|--|---|------------|
| Description: Area of top course is per roadway limits. Assumed de | pth of binder course is 2". 1 Cubic Yard of HMA ~ 2.025 tons. | |
| Calculations: | | |
| Total Roadway Area (Inside curb line) - Total Ir | sternal Turnaround Area = Total Roadway * (2/12)(ft) / 27 CF/CY | |
| 136085 SQFT - | 67930 SQFT = (68155 SQFT * 0.16667) / 27 = 420.7 CY | |
| | Converted = 851.9 TONS | |
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| | Item Total: 851.9 TON | |

Say: 900

TON

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Item: 402 | 2.258902 | | F9 Binder H | MA, 80 series cor | npaction | (2" Thickness) | Units: TON |
|--------------------|--------------------------|---------|-----------------------------------|---------------------|-------------|------------------------|------------|
| escription: | se is per roadway limits | Δeeumed | depth of binder course is 2". | 1 Cubic Vard of HM. | ^ ~ 2 N25 t | tone | |
| ed of billider occ | 30 13 poi 1000may | Addinou | depth of billider obtailed to 2 . | T Gubic Tura of F | 1.020 | ions. | |
| alculations: | | | | | | | |
| Total Roadway | Area (Inside curb line) | - To | otal Internal Turnaround Area | = Total Ro | adway | * '12)(ft) / 27 CF/CY | |
| | 136085 SQFT | - | 67930 SQFT | = (68155 | SQFT | * 0) / 27 = 420.7 CY | |
| | | | | | | Converted = 851.9 TONS | İ |
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| | | | Item Total: 8 | 351.9 TON | | | |
| | | | Say: | 900 TON | | | |
| | | | | | | | |

75 State St, Suite 701 Boston, MA 02109 PHONE: (617) 452-6000

| Item: 402.376902 | F9 Base HMA, 60 Series Compaction (4" Thickness) | Units: TON |
|---|--|------------|
| Description: Area of base course is per roadway limits. Assumed depth of l | base course is 4". 1 Cubic Yard of HMA ~ 2.025 tons. | |
| Calculations: | | |
| Total Roadway Area (Inside curb line) - Tot | ral Internal Turnaround Area = Total Roadway * (4/12)(ft) / 27 CF/CY | |
| 136085 SQFT - | 67930 SQFT = (68155 SQFT * 0.33333333) / 27 = 841.42 CY | |
| | Converted = 1703.88 TONS | |
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| | Item Total: 1,703.9 TON | |
| | Say: 1,800 TON | |
| | 349. 1,000 | |

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

 Project:
 Byram River Rte 1 Bridge Alternatives

 Sheet No:
 10
 of
 22

 Calculated By:
 Date:
 Date:

 Checked By:
 Date:
 Date:

| Item: 520.0900001 | 5 | Saw Cuttin | g Asphalt Cor | ncrete | Units: LF | |
|--|------------------------------|---------------|---------------|--------|-----------|--|
| Description: Assumed sawcutting required at roadway lim | its of project and HMA parki | ng lots to me | eet existing. | | | |
| Calculations: | | | | | | |
| Limits of Work (ft) + | HMA Parking Lots (ft) | | | | | |
| 210 + | 635 | = | 845 LF | | | |
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| | Item Total: | 845.0 | LF | | | |
| | Say: | 900 | LF | | | |

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Item: 520.5 | | Saw | ving Concrete | | Units: | LF |
|---|-----------------------|------|---------------|---|--------|----|
| Description: Assumed sawcutting required at sidewalk limits of | [;] project. | | | | | |
| Calculations: | | | | | | |
| Sidewalk Limit Sawcutting Length (| (LF) | | | | | |
| 64 LF | | | | | | |
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| | Item Total: | 64.0 | LF | | | |
| | Say: | 64 | LF | | | |
| L | | | | | | |

Boston, MA 02109

75 State St, Suite 701 PHONE: (617) 452-6000

| Project: | | Byram River Rte 1 Bridge Alternatives | | | | | | |
|----------------|-----|---------------------------------------|----|---|--|--|--|--|
| Sheet No: | 12 | of | 22 | _ | | | | |
| Calculated By: | | Date: | | _ | | | | |
| Checked By: | | Date: | | _ | | | | |
| Total: | 500 | CY | | | | | | |

| Item: 608.0101 | _ | Concrete Side | ewalks and Driveways | Units: CY |
|---|---------------------------|------------------------|---|--|
| Description: All existing sidewalks and driveways in the paverage. | roject area to be removed | d and replaced with ne | ew. Assumed concrete depth of 4" for Sid | ewalk, 6" for Driveway. Therefore used 5" as |
| Calculations: | | | | |
| Sidewalk, Driveway Width (ft) | * 2 * Pr | oject Length (accordir | ng to alignment lengths and assuming 2 si | dewalks) (ft) |
| 8 | * 2 * (| Hillside Ave 550 | Turnaround West + 150 + | Putnam Ave 200 + |
| | 4 | West Putnam Ave | Turnaround East + 125) | 0.4167 ft = 11167 CF / 27 CF/CY |
| | | | , | Converted = 413.58 CY |
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| | Item Total: | 413.6 C | ·Υ | |
| | Say: | 500 C | Υ | |
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75 State St, Suite 701
Boston, MA 02109
PHONE: (617) 452-6000

| Item: 68 | 88.01 | Wł | nite Preformed Reflect Pave | Units: LF | | |
|---|---|-----|-----------------------------|-----------|---------|--|
| Description: New striping for tra | avel lane and shoulder delineation (white | 9). | | | | |
| Calculations: | | | | | | |
| Bridge | Dashed White Center Line (LF) | + | Solid White Edge Line (LF) | | | |
| North | 550 | + | 1100 | = | 1650 LF | |
| South | 310 | + | 900 | = | 1210 LF | |
| | | | | Total | 2860 LF | |

Item Total: 2,860.0 LF

Say: 2,900 LF

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Item: 688.02 | | Yellow P | Preformed Reflect Pavement Stripes | Units: LF |
|---|------------|--------------------------|------------------------------------|-----------|
| scription: v striping for Gore Lines | and Double | yellow center line. | | |
| culations: | | | | |
| Gore Lines | + | Double Yellow Centerline | | |
| 800 | + | 300 = | 1100 LF | |
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| | | Item Total: | 1,100.0 LF | |
| | | Say: | 1,100 LF | |

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

| Item: 999.1 | Utility Adjustments (Includes Drainage, Sewer, Util Poles, Water) | Units: | EA |
|---------------|---|--------|----|
| 1101111 00011 | othity Adjustinents (includes Diamage, Jewel, Oth i Oles, Water) | • | |

Description:

Includes only the adjustment of existing utility structures such as CBs, Gate valves, Hydrants, Manholes. Cost is average of these items. This does not include replacement of any utilities.

Calculations:

Approximate number of each utility

CBs 10

Hydrants 1

Valves 10

Manholes 10

Utility Poles 3

Total = 34 EA

Item Total: 34.0 EA

Say: 34 EA

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

Project: Byram River Rte 1 Bridge Alternatives Sheet No: of Calculated By: Date: Checked By: Date: Total: 1,224,800 LS

| Item: 999.2 | Bridge Replacement - North - Option 1 - Box Beams | Units: LS |
|---------------------------------|--|-----------|
| scription: | | |
| e attached 'PRELIMINARY COST ES | STIMATE WORKSHEET (NEW AND REPLACEMENT BRIDGES)' for BIN 1000121 | |
| | | |
| Iculations: | | |
| iculations. | | |
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| | Item Total: ######## LS | |
| | Say: 1,224,800 LS | |
| | 0 4 004 000 1 0 | |

75 State St, Suite 701 Boston, MA 02109 PHONE: (617) 452-6000

| Item: 999.3 | Bridge Replacement - South - Option 1 - Box Be | eams Units: LS |
|---|--|----------------|
| Description: | | |
| See attached 'PRELIMINARY COST ESTIMATE | E WORKSHEET (NEW AND REPLACEMENT BRIDGES)' for BIN | V 1000122 |
| Calculations: | | |
| Calculations: | | |
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| | Item Total: 1,478,399.0 LS | |
| | Say: 1,478,400 LS | |

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

Subtotal =

Converted =

4935 CF / 27 CF/CY

183 CY

| Item: 999.4 | | ning Wa | II | | Uı | nits: CY | | |
|---------------------------------------|------------------------------|---------------|------------------------|-----------|---------------------|----------|----------------------|------------------|
| Description: wo retaining walls ne | ar North Bridge (on Hillside | Ave) to accor | nmodate grade change. | Cost is b | ased on Item 555.01 | 05 - Con | crete for Structures | s, Class A (CY). |
| | | | | | | | | |
| Calculations: | | | | | | | | |
| Wall Location | Wall Length (ft) | * W | /all depth (Stem) (ft) | * | Wall Width (ft) | | + | |
| North Side | 180 | | 4 | | 3.00 | | | |
| South Side | 52 | | 4 | | 2.5 | | | |
| | Footing Length (ft) | * | Footing depth (ft) | * | Footing width (ft) | = | Total | |
| North Side | 180 | | 2 | | 5 | = | 3960 CF | |
| South Side | 52 | | 2 | | 5 | = | 975 CF | |

Item Total: 182.8 CY

Say: 200 CY

75 State St, Suite 701 Boston, MA 02109

PHONE: (617) 452-6000

 Project:

 Sheet No:
 19
 of
 22

 Calculated By:
 Date:
 Date:

 Checked By:
 Date:
 Date:

 Total:
 10,000
 LS

| Item: 999.5 | 5 | | | | | | | | Lands | caping | | | | Units: | LS |
|-----------------------------------|---------|-------------|-----------|--------------|-------------|--------|------|-----------|-----------|----------------------|---------------------|---------|-----------------------|------------|----|
| escription: cludes Loam, seed, | mulch | and any oth | ner lands | scaping item | required to | accomr | noda | ite grade | e change. | Cost is a co | ombination of Seed, | mulch a | and continger | ncy items. | |
| alculations: | | | | | | | | | | | | | | | |
| Area o | f Lands | caping (per | Clearin | g and Grubb | ping) | / | 9 | SQFT | /SQYD | | | | | | |
| | 0.25 | Acres | = | 10890 S | GQFT | 1 | 9 | = | 1210 | SQYD | | | | | |
| Assume | 10 | Shrubs | | | | | | = | 10 | Shrubs | | | | | |
| Assume | 5 | Trees | | | | | | = | 5 | Trees | | | | | |
| | | | | | | | | | | Items | | | | | |
| | | | | | | | | | | 209.1003 | | = | | 66 / SQYD | |
| | | | | | | | | | | 611.0412 611.0111 | Shrubs Trees | = | \$ 90.6 \$ 1,000.6 | 60 / EA | |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | Lump Sum Cost | = | \$ 6,704.6 | 60 | |
| | | | | | | | | | | | Say | , | \$ 10,000.0 | 00 | |
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| | | | | Γ | Item | Total: | 10. | 000.0 | LS | | | | | | |
| | | | | | | | | | | | | | | | |
| | | | | | | Say: | 10 | .000 | LS | I | | | | | |

75 State St, Suite 701
Boston, MA 02109
PHONE: (617) 452-6000

| Description: Assumed 10% of total cost. Calculations: | |
|--|--|
| Calculations: | |
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| Item Total: 0.0 LS | |
| Say: 0 LS | |

| PRELIIVIINA | ARY COST ESTIMATE WC | KKSHEET | (NEW AND | KEPLACEIVIENT | BRIDGES |) | |
|-------------------------|------------------------------|----------|--------------|------------------|--------------|----------------|------------|
| P.I.N | B.I.N 1000121 | PS&E | | Anticipated Y | ear of Coi | nstruction | 2017 |
| Bridge Name: | Route 1 | | Over | Byram River | | | |
| Prepared by: | CDM Smith | | date: | 8/26/16 | | | |
| Number of Spans: | 1 | | Width (ft): | 49.17 | OR (m): | | (Width of |
| Span: | 1 | | | | | | (Length a |
| Length of Spans(ft): | 90 | | | | | | |
| OR Spans (m): | | | | | | | |
| Superstructure type: | PC box beam | | Radius (ft): | 0 | OR (m):[| |] |
| Abutment type: | solid cantilever | Ave | erage Abutm | ent Height (ft): | 8 | OR (m): | |
| Foundation type: | piles in poor soil | skew | in degrees: | 15 | | | |
| Length of wingv | valls > 60 ft: 0 | OR (m): | | Average ww h | neight ft:[| 8 | |
| Number of | Cofferdams 2 | water | depth, ftg b | ottom to OHW: | Abutments | in 4 ft to 6 f | t of water |
| Over Roadway | Height (ft): 12 | OR (m): | | (From roadway | to botto | m of culve | rt) |
| Bottor | m Angle (ft): 80 | OR (m): | | (Length of barr | el for culv | ert & 3-sio | ded frame |
| Work Zone Tra | ffic Control: on twin bridge | | | Painted: | no | Z | |
| Shoulder Break | Area (sq ft): 6516 | | | | | | |
| 1A) Base (\$/sq ft sho | oulder break area) | \$128 | | 2 | | | |
| 1B) Culverts & Three | sided frames | \$0 | | for openings 20 |) ft to 40 f | ft | |
| 2) Foundations | | \$20 | | | | | |
| 3) Abutments | | \$0 | | | | | |
| 4) Cofferdams | | \$2 | | | | | |
| 5) Span Adjustment: | | \$8 | | | | | |
| 6) Curved Girders | | \$0 | | | | | |
| 7) Long Wingwalls | | \$0 | | | | | |
| 8) Stage Construction | | \$0 | | | | | |
| 9) Miscellaneous | | \$0 | | | | | |
| Total Cost \$/sq ft sho | ulder break area | \$158 | X SB Area | 6516 | = | \$: | 1,028,340 |
| | | ******** | | Remove Existing | g Bridge: | *** | \$50,000 |
| | | | | Detour o | _ | | \$50,000 |
| | | Char | nnel work or | other slope pro | tection: | | \$0 |
| | | | Utilities, | Asthetics, or MS | SE Walls: | | \$0 |
| | | | | Ov | erhead: | | \$15,000 |
| last updated | | | | 3% Inflation p | er Year: | 1 - 2 | \$34,300 |
| 12/17/15 | (Includes 4% Mobiliztion | n) | TOTAL ESTI | MATED BRIDGE | SHARE: | \$: | 1,224,746 |

| PRELIMII | NARY COST ESTIMATE WO | DRKSHEET | (NEW AND | REPLACEMENT | BRIDGES |) | |
|------------------------|--------------------------------|----------|---------------|------------------|-------------|-----------------|------------|
| P.I.N | B.I.N 1000122 | PS&E | | Anticipated Y | ear of Cor | struction | 2017 |
| Bridge Name | e: Route 1 | | Over | Byram River | | 8 | _ |
| Prepared b | y: CDM Smith | | date: | 8/26/16 | | | |
| Number of Spans: | 1 | | Width (ft): | 59.17 | OR (m): | 120 Y, Y, | (Width of |
| Spar | n: 1 | | | | | | (Length al |
| Length of Spans(ft | 92 | | | | | | |
| OR Spans (m |): | | | | | | |
| Superstructure type | e: PC box beam | | Radius (ft): | 0 | OR (m): | GE YA | |
| Abutment type | e: solid cantilever | Ave | rage Abutm | ent Height (ft): | 8 | OR (m): | |
| Foundation type | e: piles in poor soil | skew | in degrees: | 15 | | | |
| Length of wing | gwalls > 60 ft: 0 | OR (m): | | Average ww h | neight ft:[| 8 | |
| Number o | of Cofferdams 2 | water o | lepth, ftg bo | ottom to OHW: | Abutments | in 4 ft to 6 ft | of water |
| Over Roadwa | ay Height (ft): 12 | OR (m): | | (From roadway | y to botto | m of culve | ert) |
| Botto | om Angle (ft): 80 | OR (m): | | (Length of barr | el for cul | ert & 3-si | ded frame |
| Work Zone Tr | raffic Control: on twin bridge | | | Painted: | no | | |
| Shoulder Brea | k Area (sq ft): 7841 | | | | | | |
| 1A) Base (\$/sq ft sl | houlder break area) | \$128 | | | | | |
| 1B) Culverts & Three | e sided frames | \$0 | | for openings 20 | 0 ft to 40 | ft | |
| 2) Foundations | | \$20 | | | | | |
| 3) Abutments | | \$0 | | | | | |
| 4) Cofferdams | | \$2 | | | | | |
| 5) Span Adjustment: | | \$9 | | | | | |
| 6) Curved Girders | | \$0 | | | | | |
| 7) Long Wingwalls | | \$0 | | | | | |
| 8) Stage Constructio | n | \$0 | | | | | |
| 9) Miscellaneous | | \$0 | | | | | |
| Total Cost \$/sq ft sh | oulder break area | \$158 | X SB Area | 7841 | = | \$1 | 1,240,133 |
| | | | F | Remove Existing | g Bridge: | | \$75,000 |
| | | | | Detour o | r WZTC: | | \$50,000 |
| | | Chan | | other slope pro | | | \$0 |
| | | | Utilities, A | Asthetics, or MS | | | \$0 |
| 765 si 50 | | | | | erhead: | | \$15,000 |
| last updated | | | | 3% Inflation p | - | | \$41,404 |
| 12/17/15 | (Includes 4% Mobiliztion | i) 7 | OTAL ESTIN | MATED BRIDGE | SHARE: | \$1 | ,478,399 |