## Memo

Date:	Tuesday, April 12, 2016
Project:	Passaic Tidal Closure Structure
To:	John Dromsky-Reed
From:	Wes Jacobs Bogdan Bogdanovic
Subject:	Preliminary Design and Cost Study of the Passaic River Closure Gate Structures in Support of the Passaic River Ecosystem Restoration Feasibility Studies Conducted under Indefinite Delivery Contract IDC-PL-3005: W912DS-11-D-0008

This memorandum presents the results of our preliminary (planning level) designs and cost study of the Passaic River closure gate structures in the tidal areas of Kearney, Newark, and Harrison, NJ.

### 1. INTRODUCTION

This study addresses the design of closure gates in typical reaches along the Passaic River extending from Kearny to Harrison, NJ. The design and cost element defined herein represents a 30-percent level conceptual design using the latest revision inventory document of the closure gates titled "Passaic River-Final Closure Gate Inventory" dated March 10, 2016 (see Appendix A). The closure gates were grouped into several different categories based on gate openings, heights and types. The gate types used were predominantly swing gates with the exception of roller gates for openings of 50 feet or larger. The gates are assumed a mix of closures to span railroads, highways and pedestrian crossings.

Through coordination with technical staff from AECOM, as well as standard engineering practice, the 30% design includes four basic load cases which are loadings that typically control floodwall/closure gate structures designs. A full array of load cases will need to be investigated in the final design phase. The load cases included in the 30% design are:

- Construction + Wind: Dead load of the concrete monolith and steel gate, a conservative wind load
  of 50 psf, no earthen backfill, no uplift, no construction surcharge. A 33% overstress is permitted
  for this load case.
- Flood stage at still water (SWL) at 2 feet below top of gate structure with debris impact loading of 500lbs/ft applied at the SWL. A 33% overstress is permitted for this load case.
- Flood stage at water to top of gate (TOG). Wave force is not included. A 33% overstress is permitted for this load case.
- Flood stage at SWL at 2 feet below top of gate structure. A 0% overstress is permitted for this load case.

The gate members (girders, intercostals, and skin plates), concrete monolith (abutments/footings), and foundations were sized to carry these anticipated loads as mentioned above for all different gate categories which have been selected. Secondary gate features such as any hinge assemblies, connections, casters, trolleys, or hanger systems were conceptually shown based on previous similar projects and engineering judgment. Calculations were not performed to size these types of features. Wave loadings are expected to be minimal due to topographic conditions and lack of proximity/exposure

to full coastal storm surge associated with hurricanes. It is also assumed, per technical discussions, that there will be no unbalanced loading or downdrag forces seen by the gates at this level of design. This will require more in-depth analysis and can be fully vetted during later design stages. Complex pile group analysis, therefore, was not be required. Seismic forces were not considered to govern and were not applied at this level of design.

For the 30 % design effort the following codes and standards will be used, as well as the applicable portions of the HSDRRSDG (Hurricane Storm Damage Risk Reduction System Design Guidelines) and the existing project GDM:

- EM 1110-2-2705 Structural Design of Closure Structures for Local Flood Protection Projects
- EM 1110-2-2104 Strength Design for Concrete Hydraulic Structures
- EM 1110-2-2105 Strength Design for Hydraulic Steel Structures

Once the preliminary gate designs were compiled for all different gate selections, costs were developed based on the major contributing "bid" items that would typically be present in final documents such as: concrete monolith structure (abutments and footings), structural steel gate (gate overall weight plus detail factor), concrete reinforcing for monolith structure, and pile foundation (total pile length for the gates). Items such as steel embeds, seals, turnbuckles, casters, hinge assemblies, access ladders, etc. were included in the structural steel gate item. Unit prices were based on recent, similar construction projects and adjusted for any regional effects and applied to the various bid item quantities.

### 2. DISCUSSION AND RESULTS

The final closure gate inventory has 64 closure gate structures that fluctuate in gate opening width and gate height. The gate heights for all 64 closure gates were determined based on the design water elevation of 14 feet and their respective existing grade elevations. In addition, evaluations were completed for gate heights 2 feet and 4 feet above the 14 foot elevation.

All gates were grouped into several scenarios based on gate openings and heights as shown in table 1. The Kearny, Newark and Flanking areas consist of H-pile foundation whereas the Harrison area consists of concrete micro pile foundation. Any opening width equal to 10-feet or smaller was grouped with the 10-foot gate opening. The 20-foot gate opening was grouped with a series of opening widths ranging from 15 to 20 feet. The majority of opening widths in the inventory was for the 30-foot width. The 30-foot gate opening was grouped from 25 to 30 feet. The 35-foot, 40-foot, 45-foot and 50-foot gate openings were grouped individually, since their gate opening width is considered to be on the larger end of the swinging gate spectrum.

Once the gates were group as described above, the smallest gate height and the tallest gate height for each respective group was determined and a 2-feet incremental height increase was implemented starting from the minimum to the maximum gate heights. Typically gates for openings larger than 38 feet would be considered at the threshold for the swing gates. Roller gates predominantly are seen for openings larger than 38 feet. The gate opening width identified in the flanking area of the final closure structure inventory ranged from 40 to 150 feet. After further assessment of the gate openings in the flanking area, the roller gate option will not be feasible due to the limited space in this area which does not facilitate the construction of the larger concrete monolith structure. Therefore the 150 feet opening was divided into three swing gates with an opening of 50 feet. The inventory list also includes four gate widths opening of 50 feet which have been grouped together as roller gates since the vicinity permitted a

larger concrete monolith structure. The same grouping procedure described above was followed with respect to gate heights.

GATE OPENING		S'	WING	GATE	(H-Pil	e Fou	ındatio	n)			
(Feet)		GATE HEIGHTS(Feet)									
10	6	8	10	12	14	16	-	-	-		
20	5	7	9	11	13	-	-	-	-		
30	2	4	6	8	10	12	14	16	18		
35	9	11	13	15	17	-	-	-	-		
40	10	12	14	-	-	-	-	-	-		
45	5	7	9	-	-	-	-	-	-		
50	6	8	10	-	-	-	-	-	-		
GATE OPENING	SWI	NG GA	TE(Mi	cro Pil	e Fou	ndati	on, Ha	rrison	Area)		
(Feet)			(	GATE H	EIGHT	S(Fee	t)				
30	11	13	15	17	-	-	-	-	-		
40	2	4	6	8	10	12	-	-	-		
GATE		ROLLER GATE									
OPENING (Feet)		GATE HEIGHTS(Feet)									
50	10	12	14	16	_	_	-	-	_		

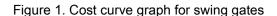
Table 1. Gate Grouping Scenarios

The structural design of the swing/roller gate includes the layout and design of the major structural elements of the concrete monolith structure and floodgate. This includes the gate steel members, the concrete gate bay walls and support columns, base slab and the pile foundations. The structural steel gate members include top and bottom girders spanning horizontally between concrete bay columns, vertical intercostal framing spaced at approximately 2 feet on center and spanning between top and bottom girders, steel skin plate spanning between the vertical intercostal, and steel cross bracing and horizontal bracing. The concrete monoliths are comprised of two concrete gate bay walls/columns on either side which are formed into the base slab and pile foundation. The concrete monoliths are supported by the pile foundations. Steel H-piles and concrete micro piles were applied during design for consistency with the floodwall team. It is assumed that each gate monolith structure will be flanked by the floodwall structures in the adjacent reaches. The floodgate drawings in Appendix B are preliminary in nature and not to be used for construction. The sections and views on the drawings are grouped as described above in table 1. Based on the gate width and heights, the design elements will vary in size, location and spacing accordingly.

The analysis of the steel gate and concrete monolith was performed based on the load cases noted in the introduction. The governing load case was typically the flood stage with water at the top of the gate. Loads were applied as hydrostatic pressures corresponding to the water surface elevations on the flood-side. A debris impact uniform loading (500lbs/ft.) was applied at the appropriate water surface elevations. The skin plate was designed as a fixed end beam spanning between the vertical intercostals and the deflection was limited to 0.4 of the thickness to ensure that the flat plate theory is applicable. The horizontal girders were designed as larger wide flange simply supported beams spanning between the bearing points on the concrete columns making them true beam elements allowing for flexural stresses.

The vertical intercostals were designed as simple beams spanning between horizontal girders. The vertical intercostals consist of a WT section welded to the skin plate and were designed as a combined section utilizing the steel skin plate as the tension flange of the total combined section. The analysis of the reinforced concrete monolith walls and columns was performed considering fixed support at the interface of the bottom of the wall and top of slab. The wall analysis considered a 1 foot unit width of the wall acting as a cantilever and connected only to the base slab. The column analysis considered half of the gate width and width of the column loading on the column acting as a cantilever and connected only to the base slab. The entire analysis for the floodgate and concrete monolith was carried out by hand calculations for one gate width and height which than an excel spreadsheet program was developed to generated the analysis design for all chosen gate scenarios listed in table 1. The calculations are provided in Appendix C.

Opinions of probable cost (using unit prices from similar, recent projects) were developed based on the results of the analysis above. The cost estimate was broken down into four items corresponding to each individual gate width and height. The four cost items are the structural steel gate, concrete monolith structure, concrete reinforcing and pile foundation with a final total project cost. The cost breakdown for all listed scenarios is provided in table format in Appendix D. In addition, compiled cost curve graphs for each gate opening width based on total project cost versus gate height to gate opening width were developed and are shown below for each gate type.



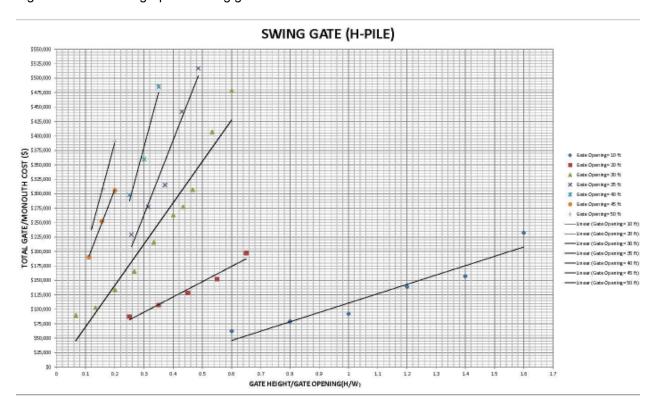
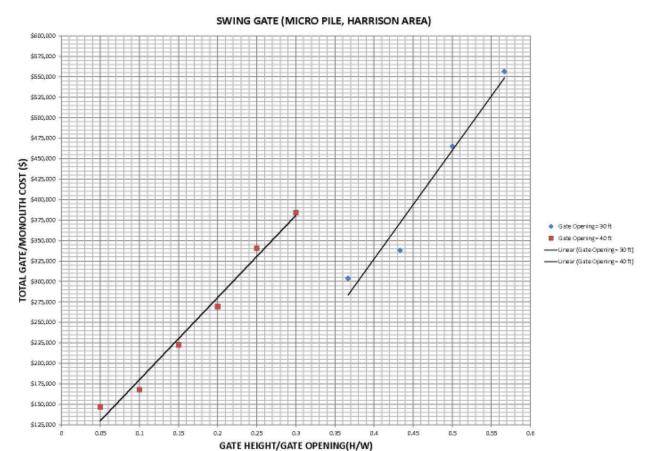
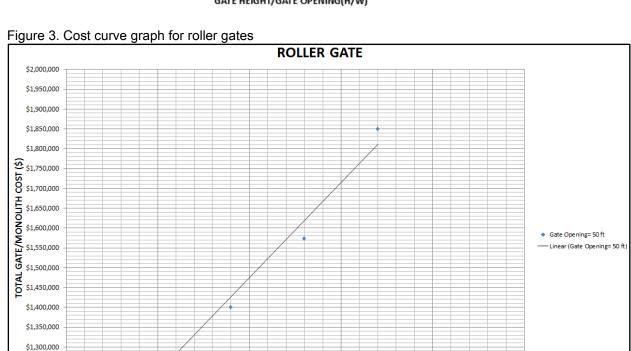


Figure 2. Cost curve graph for swing gates (micro pile, Harrison Area)





GATE HEIGHT/GATE OPENING(H/W)

\$1,250,000 \$1,200,000

0.25

The intent to these curves is to be able to achieve an approximate construction cost estimate based on the gate width opening for varying gate height to width ratio. Opinions of probable construction cost for each of the 64 gates in the inventory were developed from these cost curves and are reported in Appendix A. The closure gate costs, by reach, are reported in table 2.

Table 2. Summary of Closure Gate Costs By Project Reach

		Project Reach							
Design Water Surface Elevation <sup>1</sup>	Kearney	Newark	Harrison 1	Newark Flanking	TOTAL				
14 ft (GDM)	\$8,247,020	\$4,023,917	\$2,403,056	\$1,558,707	\$16,232,701				
16 ft (GDM + 2 ft)	\$9,896,957	\$5,402,250	\$3,242,335	\$2,108,385	\$20,649,928				
18 ft (GDM + 4 ft)	\$11,556,451	\$6,780,583	\$3,956,389	\$2,658,063	\$24,951,486				

All elevations reference the NAVD 88 vertical datum.

# Appendix A: Passaic River-Final Closure Gate Inventory and Cost

PASSAIC RIVER - FINAL CLOSURE GATE INVENTORY

Passaic River Ecosystem Restoration Feasibility Studies Conducted under Indefinite Delivery Contract IDC-PL-3005: W912DS-11-D-0008 USACE New York District

FINAL Closure Structure Inventory, 10 March 2016

### **Total Number of Closure Structures: 64**

Source Documents: GDM September 1995 - Vol II of II; AECOM e-mail transmittal,

Originator: Bogdan Bogdanovich

QC: Michael Vecchio, 12/8/15; Revision 1 - 1/19/16, Revision 2 - 3/10/16 for Closure Gate Inventory, Post-QC Revisions: 4/14/2016

Incorporation of Cost Equations: K Hayden - 3/29/2016; K Hayden Addition of micro-pile 40-ft wide gate cost equations - 0 4/21/16

QC of Cost Equations: Jaak Van den Sype, 3/31/16; Kim Hayden - 4/14/2016; Michael Vecchio - 4/21/16

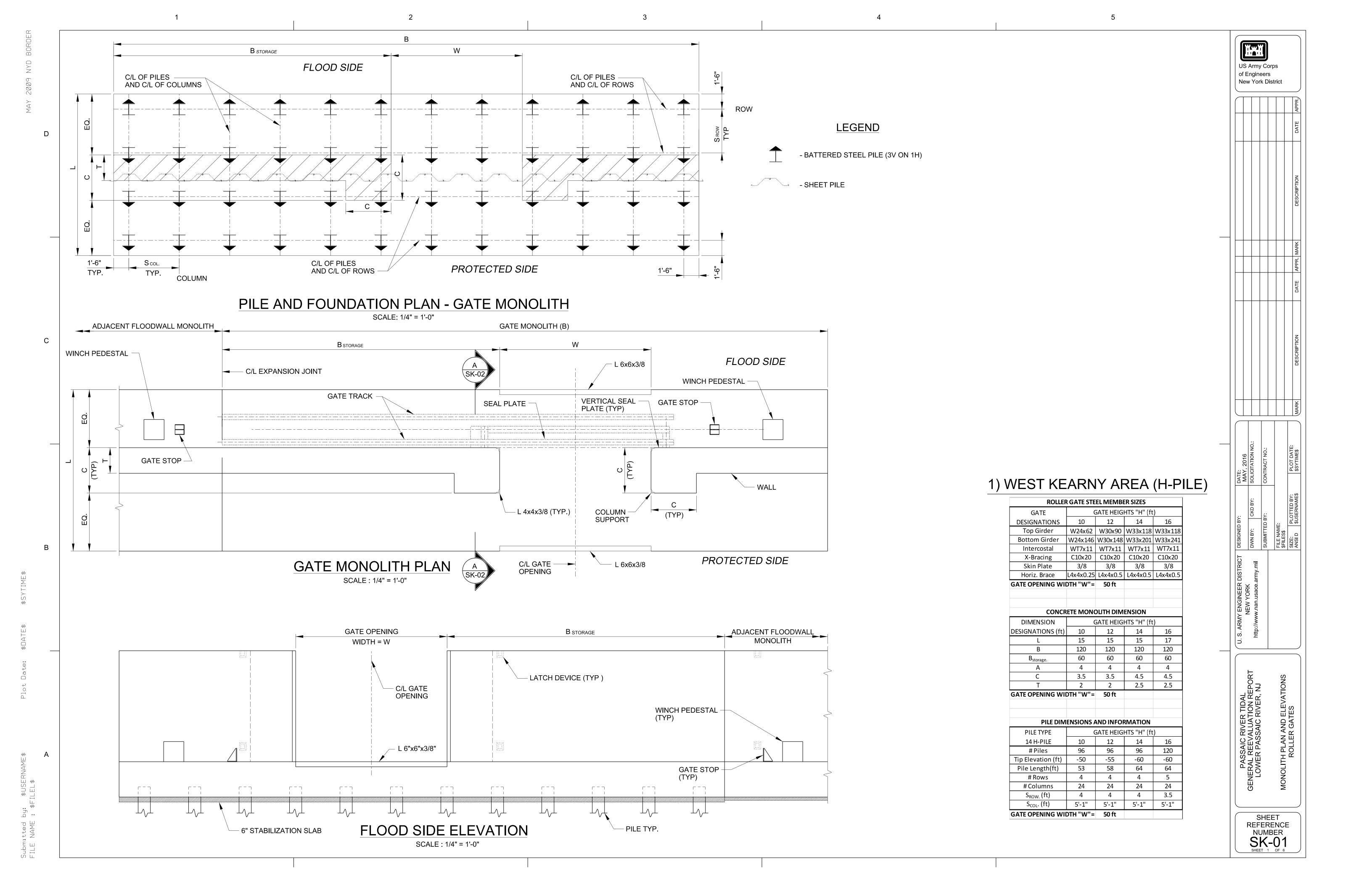
QC of Transmitted Table: Michael Murphy, 4/7/2016

				Original						Computed								
				GDM	Opening					GDM		GDM		H/O for	GDM + 2 ft	Computed		GDM + 4 ft
		GIS file description from	Gate	Height	Width (O)	Existing	2	2	2	Height (H)	H/O for	Construction	Computed GDM	GDM +2	Construction	GDM +4	H/O for	Construction
Gate No.	Revised Reach	AECOM	Type	(ft) <sup>4</sup>	(ft) <sup>4</sup>	Grade (ft)	GDM DWSE (ft) <sup>2</sup>	$GDM + 2 ft^3$	GDM + 4 ft <sup>3</sup>	(ft)	GDM	Cost⁵	+2 Height (ft)	ft	Cost <sup>5</sup>	Height (ft)	GDM+4 ft	Cost <sup>5</sup>
5	Kearny	RAILROAD CLOSURE	swing	8.0	35	8.0	14.0	16.0	18.0	6.0	0.2	\$ 97,258	8.0	0.2	\$ 171,216	10.0	0.286	\$ 245,173
	Kearny	±15 L.F. CLOSURE ±7.0' HT. T.O.L. 14.9'	swing	7.0	15	7.2	14.0	16.0	18.0	6.8	0.5	\$ 59,431	8.8	0.6	\$ 75,456	10.8	0.723	\$ 91,481
7	Kearny	±15 L.F. CLOSURE ±3.5' HT. T.O.L. 14.9'	swing	3.5	15	7.5	14.0	16.0	18.0	6.5	0.4	\$ 56,337	8.5	0.6	\$ 72,362	10.5	0.698	\$ 88,387
	Kearny	±15 L.F. CLOSURE ±3.5' HT. T.O.L. 14.9'	swing	8.5	15	8.6	14.0	16.0	18.0	5.4	0.4	\$ 47,608	7.4	0.5	\$ 63,633	9.4	0.625	\$ 79,658
	Kearny	±35 L.F. CLOSURE ±3.5' HT. T.O.L. 14.9'	swing	8.5	35	5.1	14.0	16.0	18.0	8.9	0.3	\$ 204,849	10.9	0.3	\$ 278,806	12.9	0.369	\$ 352,764
10	Kearny	±25 L.F. CLOSURE ±7.0' HT. T.O.L. 14.9'	swing	7.0	25	6.8	14.0	16.0	18.0	7.2	0.3	\$ 61,879	9.2	0.4	\$ 74,816	11.2	0.447	\$ 87,752
11	Kearny	±50 L.F. CLOSURES ±8.5' HT.	roller	8.5	50	1.6	14.0	16.0	18.0	12.4	0.2	\$ 1,465,076	14.4	0.3	\$ 1,656,990	16.4	0.328	\$ 1,848,905
12	Kearny	±50 L.F. CLOSURES ±8.5' HT.	roller	8.5	50	3.8	14.0	16.0	18.0	10.2	0.2	\$ 1,255,561	12.2	0.2	\$ 1,447,476	14.2	0.284	\$ 1,639,390
13	Kearny	±50 L.F. CLOSURES ±8.5' HT.	roller	8.5	50	2.6	14.0	16.0	18.0	11.4	0.2	\$ 1,365,937	13.4	0.3	\$ 1,557,852	15.4	0.307	\$ 1,749,766
14	Kearny	30' CLOSURE ±6.9' HT.	swing	6.9	30	7.4	14.0	16.0	18.0	6.6	0.2	\$ 124,496	8.6	0.3	\$ 181,378	10.6	0.354	\$ 238,259
15	Kearny	30' CLOSURE ±7.2' HT.	swing	7.2	30	6.0	14.0	16.0	18.0	8.0	0.3	\$ 163,383	10.0	0.3	\$ 220,265	12.0	0.400	\$ 277,147
16	Kearny	±30 L.F. CLOSURE ±8.4' HT. T.O.L. 14.9'	swing	8.4	30	3.9	14.0	16.0	18.0	10.1	0.3	\$ 222,050	12.1	0.4	\$ 278,932	14.1	0.469	\$ 335,814
17	Kearny	±30 L.F. CLOSURE ±8.4' HT. T.O.L. 14.9'	swing	8.4	30	4.0	14.0	16.0	18.0	10.0	0.3	\$ 220,416	12.0	0.4	\$ 277,298	14.0	0.467	\$ 334,180
18	Kearny	±30 L.F. CLOSURE ±8.9' LOCATION MAY	swing	8.9	30	6.0	14.0	16.0	18.0	8.0	0.3	\$ 163,630	10.0	0.3	\$ 220,512	12.0	0.400	\$ 277,394
19	Kearny	±35 L.F. CLOSURE ±8.9' HT. T.O.L. 14.9'	swing	8.9	35	0.7	14.0	16.0	18.0	13.3	0.4	\$ 366,789	15.3	0.4	\$ 440,747	17.3	0.494	\$ 514,704
20	Kearny	±50 L.F. CLOSURE ACROSS EXISTING DISC	roller	6.0	50	7.3	14.0	16.0	18.0	6.7	0.1	\$ 914,901	8.7	0.2	\$ 1,106,815	10.7	0.213	\$ 1,298,729
21	Kearny	±20 L.F. CLOSURE 5.0' HT. T.O.L. 14.9'	swing	6.0	20	5.8	14.0	16.0	18.0	8.2	0.4	\$ 124,304	10.2	0.5	\$ 150,731	12.2	0.611	\$ 177,158
21A	Kearny	±20 L.F. CLOSURE 5.0' HT. T.O.L. 14.9'	swing	6.0	20	5.2	14.0	16.0	18.0	8.8	0.4	\$ 131,638	10.8	0.5	\$ 158,066	12.8	0.639	\$ 184,493
	Kearny		roller	?	50	6.3	14.0	16.0	18.0	7.7	0.2	\$ 1,012,969	9.7	0.2	\$ 1,204,883	11.7	0.234	\$ 1,396,798
	Kearny		swing	5.0	30	12.9	14.0	16.0	18.0	1.1	0.0	\$ 83,158	3.1	0.1	\$ 96,494	5.1	0.169	\$ 119,386
	Kearny		swing	5.0	30	8.0	14.0	16.0	18.0	6.0	0.2	\$ 105,349	8.0	0.3	\$ 162,230	10.0	0.332	\$ 219,112
24	KEARNY SUBTOTA	ΔIS	34411IS	5.0	30	5.0	2 1.0	10.0	10.0	5.0	J.2	\$ 8,247,020	5.0	5.5	\$ 9,896,957	13.0	5.552	\$ 11.556.451
26	Newark	±30 L.F. CLOSURE ±7.5' HT. T.O.L. 14.9'	nul	7.5	30	4.3	14.0	16.0	18.0	9.7	0.3	\$ 210,647	11.7	0.4	\$ 267,529	13.7	0.455	\$ 324,410
	Newark		swing	6.0		6.6	14.0	16.0	18.0	7.4		\$ 146,688	9.4	0.4		11.4	0.380	\$ 260,452
27		±30 L.F. CLOSURE ±6.0' HT. T.O.L. 14.9'	swing	6.0	30 30	6.9	14.0	16.0	18.0	7.4	0.2	\$ 146,688	9.4	0.3	\$ 203,570 \$ 194,155	11.4	0.369	\$ 251,037
	Newark	±30 L.F. CLOSURE ±6.0' HT. T.O.L. 14.9'	swing	6.0	30	5.8	14.0	16.0	18.0	8.2	0.2	\$ 137,273	10.2	0.3	\$ 194,155	11.1	0.369	\$ 282,720
			swing									,						
	Newark	±30 L.F. CLOSURE ±6.0' HT. T.O.L. 14.9'	swing	6.0	30	6.9	14.0	16.0	18.0	7.1	0.2	\$ 137,158	9.1	0.3	\$ 194,040	11.1	0.369	\$ 250,922
	Newark	±30 L.F. CLOSURE ±9.0' HT. T.O.L. 14.9'	swing	9.0	30	4.9	14.0	16.0	18.0	9.1	0.3	\$ 194,953	11.1	0.4	\$ 251,835	13.1	0.437	\$ 308,716
	Newark	±30 L.F. CLOSURE ±7.5' HT. T.O.L. 14.9'	swing	7.5	30	6.4	14.0	16.0	18.0	7.6	0.3	\$ 152,538	9.6	0.3	\$ 209,419	11.6	0.387	\$ 266,301
	Newark	±30 L.F. CLOSURE ±8.0' HT. T.O.L. 14.9'	swing	6.0	30	3.6	14.0	16.0	18.0	10.4	0.3	\$ 231,966	12.4	0.4	\$ 288,847	14.4	0.480	\$ 345,729
	Newark	±30 L.F. CLOSURE ±5.5' HT. T.O.L. 14.9'	swing	5.5	30	5.8	14.0	16.0	18.0	8.2	0.3	\$ 170,512	10.2	0.3	\$ 227,394	12.2	0.408	\$ 284,276
	Newark	±30 L.F. CLOSURE ±5.5' HT. T.O.L. 14.9'	swing	5.5	30	8.8	14.0	16.0	18.0	5.2	0.2	\$ 83,994	7.2	0.2	\$ 140,876	9.2	0.307	\$ 197,757
	Newark	±10 L.F. CLOSURE ±10.0' HT. T.O.L. 14.9'	swing	10.0	10	3.6	14.0	16.0	18.0	10.4	1.0	\$ 116,652	12.4	1.2	\$ 148,977	14.4	1.436	\$ 181,301
	Newark	±30 L.F. CLOSURE ±9.0' HT. T.O.L. 14.9'	swing	9.0	30	5.8	14.0	16.0	18.0	8.2	0.3	\$ 168,277	10.2	0.3	\$ 225,159	12.2	0.405	\$ 282,040
	Newark	±30 L.F. CLOSURE ±9.0' HT. T.O.L. 14.9'	swing	9.0	30	4.7	14.0	16.0	18.0	9.3	0.3	\$ 201,141	11.3	0.4	\$ 258,023	13.3	0.444	\$ 314,905
	Newark	±30 L.F. CLOSURE ±9.0' HT. T.O.L. 14.9'	swing	9.0	30	6.2	14.0	16.0	18.0	7.8	0.3	\$ 159,251	9.8	0.3	\$ 216,133	11.8	0.395	\$ 273,015
41	Newark	±30 L.F. CLOSURE ±9.0' HT. T.O.L. 14.9'	swing	9.0	30	5.4	14.0	16.0	18.0	8.6	0.3	\$ 181,027	10.6	0.4	\$ 237,909	12.6	0.420	\$ 294,791
42	Newark	PROVIDE 1-10' CLOSURE 2-4' CLOSURE	swing		10	5.4	14.0	16.0	18.0	8.6	0.9	\$ 88,451	10.6	1.1	\$ 120,775	12.6	1.262	\$ 153,100
	Newark	PROVIDE 1-10' CLOSURE 2-4' CLOSURE	swing		4	8.5	14.0	16.0	18.0	5.5	1.4	\$ 61,365	7.5	1.9	\$ 86,333	9.5	2.381	\$ 111,301
	Newark	PROVIDE 1-10' CLOSURE 2-4' CLOSURE	swing		4	9.6	14.0	16.0	18.0	4.4	1.1	\$ 47,397	6.4	1.6	\$ 72,365	8.4	2.101	\$ 97,333
	Newark	±10 L.F. CLOSURE ±6.0' HT. T.O.L. 14.9'	swing	6.0	10	7.7	14.0	16.0	18.0	6.3	0.6	\$ 50,601	8.3	0.8	\$ 82,926	10.3	1.028	\$ 115,250
48	Newark	±10 L.F. CLOSURE ±6.0' HT. T.O.L. 14.9'	swing	6.0	10	6.6	14.0	16.0	18.0	7.4	0.7	\$ 68,304	9.4	0.9	\$ 100,628	11.4	1.137	\$ 132,953
	Newark	20 L.F. CLOSURE ±6.0' HT.	swing	6.0	20	7.2	14.0	16.0	18.0	6.8	0.3	\$ 105,350	8.8	0.4	\$ 131,778	10.8	0.539	\$ 158,205
49A		±10 L.F. CLOSURE ±6.0' HT. T.O.L. 14.9'	swing	6.0	10	8.0	14.0	16.0	18.0	6.0	0.6	\$ 46,120	8.0	0.8	\$ 78,444	10.0	1.000	\$ 110,769
	Newark	±10 L.F. CLOSURE ±6.0' HT. T.O.L. 14.9'	swing	6.0	10	6.8	14.0	16.0	18.0	7.2	0.7	\$ 64,901	9.2	0.9	\$ 97,225	11.2	1.116	\$ 129,550
51	Newark	30' L.F. CLOSURE ±4.5' HT.	swing	4.5	30	6.7	14.0	16.0	18.0	7.3	0.2	\$ 144,461	9.3	0.3	\$ 201,343	11.3	0.378	\$ 258,225
52	Newark	30' L.F. CLOSURE ±4.5' HT.	swing	4.5	30	6.5	14.0	16.0	18.0	7.5	0.3	\$ 150,648	9.5	0.3	\$ 207,530	11.5	0.385	\$ 264,412
53	Newark	20' CLOSURE ±4' HT.	swing	4.0	20	9.6	14.0	16.0	18.0	4.4	0.2	\$ 74,104	6.4	0.3	\$ 100,531	8.4	0.421	\$ 126,958
	NEWARK SUBTO	TALS										\$ 4,023,917			\$ 5,402,250			\$ 6,780,583
53A	Harrison 1	30' L.F. CLOSURES ±6.4' HT. & ±7.4' HT.	swing		40	7.5	14.0	16.0	18.0	6.5	0.2	\$ 243,026	8.5	0.2	\$ 293,132	10.5	0.263	\$ 343,237
	Harrison 1	30' L.F. CLOSURES ±6.4' HT. & ±7.4' HT.	swing	6.4	40	7.9	14.0	16.0	18.0	6.1	0.2	\$ 233,598	8.1	0.2	\$ 283,704	10.1	0.253	\$ 333,810
55	Harrison 1	30' CLOSURES ±8.4' HT. NEED TO CHECK	swing	7.4	30	5.1	14.0	16.0	18.0	8.9	0.3	\$ 189,288	10.9	0.4	\$ 277,729	12.9	0.429	\$ 366,170
56	Harrison 1	30' L.F. CLOSURES ±6' HT.	swing	7.4	30	4.3	14.0	16.0	18.0	9.7	0.3	\$ 227,249	11.7	0.4	\$ 315,690	13.7	0.458	\$ 404,131
57	Harrison 1	30' L.F. CLOSURES ±6' HT.	swing	8.4	30	3.9	14.0	16.0	18.0	10.1	0.3	\$ 244,463	12.1	0.4	\$ 332,904	14.1	0.471	\$ 421,345
58	Harrison 1	30' L.F. CLOSURES ±6' HT.	swing	6.0	30	1.4	14.0	16.0	18.0	12.6	0.4	\$ 354,728	14.6	0.5	\$ 443,169	16.6	0.554	\$ 531,610
59	Harrison 1	30' L.F. CLOSURES ±6' HT.	swing	6.0	30	4.1	14.0	16.0	18.0	9.9	0.3	\$ 234,237	11.9	0.4	\$ 322,678	13.9	0.463	\$ 411,119
59A	Harrison 1		swing		40	9.3	14.0	16.0	18.0	4.7	0.1	\$ 196,894	6.7	0.2	\$ 247,000	8.7	0.217	\$ 297,105
	Harrison 1		swing		40	8.8	14.0	16.0	18.0	5.2	0.1	\$ 211,145	7.2	0.2	\$ 261,251	9.2	0.231	\$ 311,357
	Harrison 1		swing		40	6.5	14.0	16.0	18.0	7.5	0.2	\$ 268,428	9.5	0.2	\$ 318,534	11.5	0.288	\$ 368,639
59D	Harrison 1		swing		40	14.0	14.0	16.0	18.0	0.0	0.0		2.0	0.1	\$ 146,546	4.0	0.100	\$ 167,867
	HARRISON 1 SUB	TOTALS										\$ 2,403,056			\$ 3,242,335			\$ 3,956,389
60		- Western most section (1)	swing	4	45	12.0	14.0	16.0	18.0	2.0	0.0	\$ 105,033	4.0	0.1	\$ 162,756	6.0	0.133	\$ 220,478
	Newark Flanking		swing	4	45	9.4	14.0	16.0	18.0	4.6	0.1	\$ 179,386	6.6	0.1	\$ 237,109	8.6	0.191	\$ 294,832
	Newark Flanking		swing	4	45	9.6	14.0	16.0	18.0	4.4	0.1	\$ 173,077	6.4	0.1	\$ 230,799	8.4	0.186	\$ 288,522
	Newark Flanking		swing	8	30	6.0	14.0	16.0	18.0	8.0	0.3	\$ 163,365	10.0	0.3	\$ 220,246	12.0	0.400	\$ 277,128
		(3) 50-foot swing gates in lieu of 150-foo	swing	8	50	8.3	14.0	16.0	18.0	5.7	0.1	\$ 682,255	7.7	0.2	\$ 908,021	9.7	0.194	\$ 1,133,787
	Newark Flanking		swing	10	40	4.7	14.0	16.0	18.0	9.3	0.2	\$ 255,592	11.3	0.3	\$ 349,454	13.3	0.333	\$ 443,316
	NEWARK FLANKI											\$1,558,707			\$2,108,385			\$2,658,063
	TOTALS											\$16,232,701			\$20,649,928			\$24,951,486
	ded by AECOM	ı						·				+-3,-3-,701		ı	, , J . J , J E U	<b>.</b>	ı	Ţ,551,.50

<sup>1</sup> Data provided by AECOM 2 Provided by AECOM 3 Per scope of work.

<sup>4</sup> Taken from 1/27/90 GDM drawings.
5 Construction includes the following materials, exclusively: structural steel gate; concrete monolith structure; concrete reinforcing; piles foundation

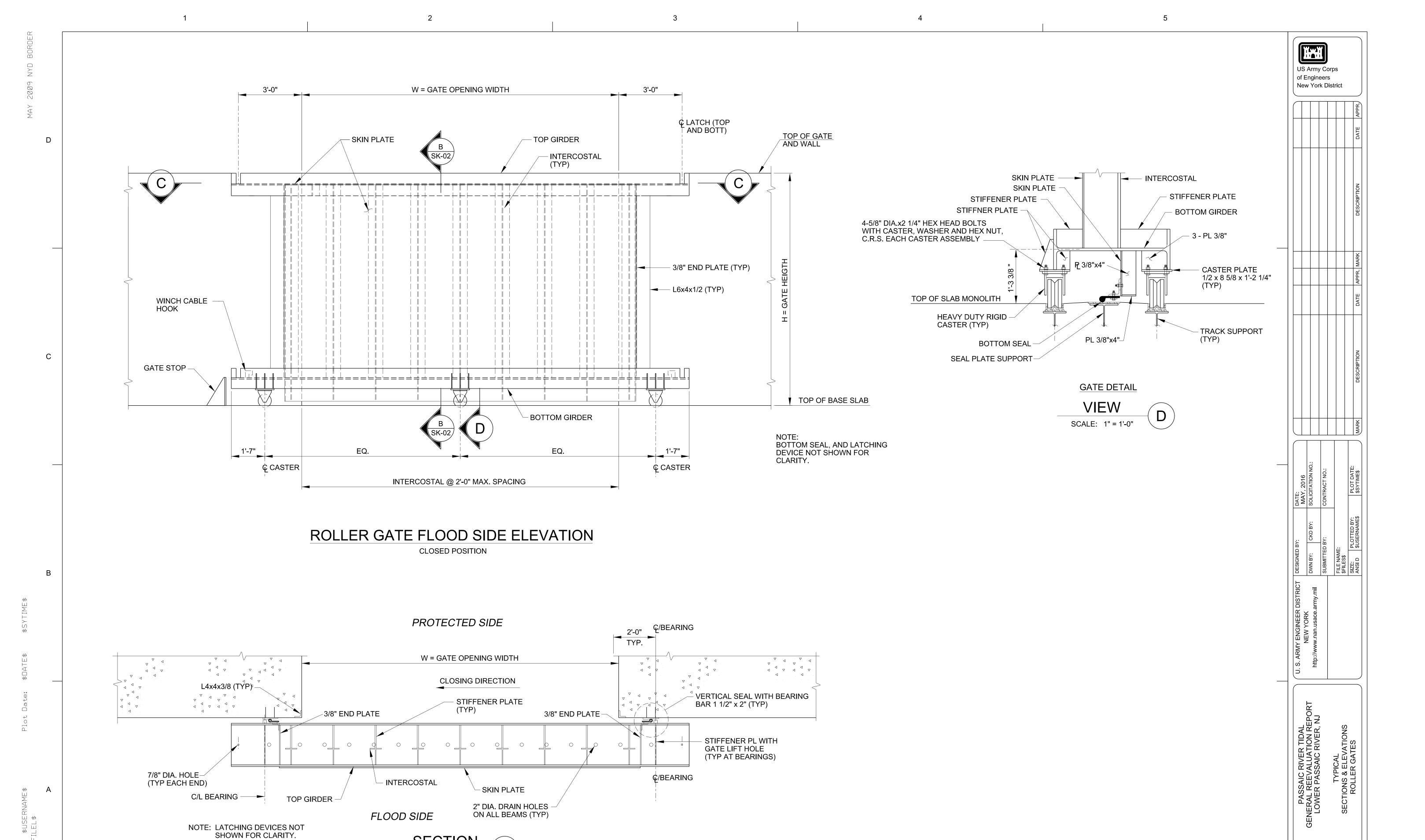
# Appendix B: Typical Flood-Gate and Monolith Concrete Structure Drawings



HYM US Army Corps of Engineers New York District

PASSAIC RIVER TIDAL
GENERAL REEVALUATION REPORT
LOWER PASSAIC RIVER, NJ
TYPICAL MONOLITH
AND ROLLER GATE
SECTIONS

SHEET
REFERENCE
NUMBER
SK-02
SHEET 2 OF 6

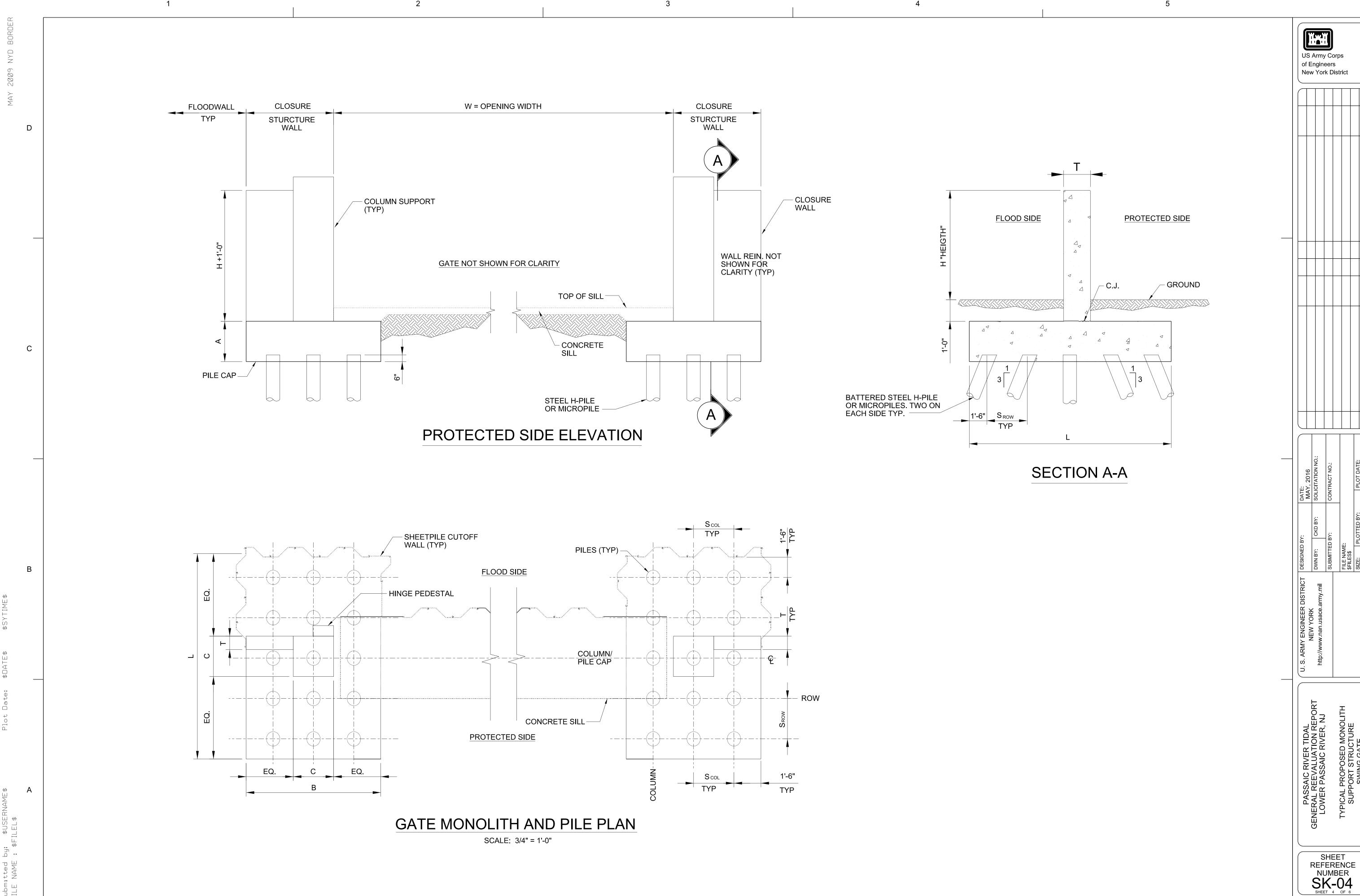


SHEET
REFERENCE
NUMBER
SK-03
SHEET 3 OF 6

SECTION

SCALE: 1/2" = 1'-0"

Submitted FILE NAME



HAH US Army Corps of Engineers New York District В W = GATE OPENING WIDTH GATE LATCH 9
TOP AND BOTTOM GIRDER INTERCOSTAL X- BRACING -FACE OF WALL TOP OF GATE TOP GIRDER X-BRACING UPPER HINGE INTERCOSTAL HORIZONTAL BRACE - SIDE PLATE (TYP) PROTECTED FLOOD SIDE HORIZONTAL BRACING - END PLATE (TYP) - LOWER HINGE 1/2" STIFFENER PLATE (TYP) SKIN PLATE -CONTINUOUS SEAL - BOTTOM GIRDER BOTTOM GIRDER 1/2" STIFFENER PL (TYP) TOP OF SILL ု် LOWER HINGE INTERCOSTALS @ 2'-0" SPACING MAX. SECTION B-B  $\left(\mathsf{B}\right)$ FLOOD SIDE ELEVATION SCALE: 1/4" = 1'-0" SCALE: 3/4" = 1'-0" FACE OF WALL FACE OF WALL-PROTECTED SIDE SKIN PLATE - INTERCOSTAL DRAIN HOLES<u>c</u> BOTTOM GIRDER PASSAIC RIVER TIDAL GENERAL REEVALUATION REPORT LOWER PASSAIC RIVER, NJ TYPICAL PROPOSED GATE PLAN AND ELEVATION SWING GATE – 2" DIA DRAIN HOLES-TOP & BOT. GIRDERS (TYP) — X-BRACING (TYP) FLOOD SIDE **SECTION A-A** SCALE: 3/4" = 1'-0" SHEET
REFERENCE
NUMBER
SK-05
SHEET 5 OF 6

## EAST/WEST OF KEARNY, NEWARK AND HARRISON AREA (H-PILE)

	300110		EEL MEMBI			
GATE		(	GATE HEIGI	HTS "H" (ft	)	<u> </u>
DESIGNATIONS	6	8	10	12	14	16
Top Girder	Wx8x13	W8x13	W10x12	W12x16	W14x22	W14x22
Bottom Girder	W8x18	W8x18	W10x22	W12x26	W14x30	W14x34
Intercostal	WT3x6	WT3x6	WT4x6.5	WT5x7.5	WT6x8	WT7x11
X-Bracing	C10x20	C10x20	C10x20	C10x20	C10x20	C10x20
Skin Plate	5/16	5/16	5/16	5/16	6/16	6/16
Horiz. Brace	L4x4x0.25	L4x4x0.5	L4x4x0.5	L4x4x0.5	L4x4x0.5	L4x4x0.5
GATE OPENING W	IDTH "W"=	10 ft				
	CONC	RETE MON	OLITH DIM	ENSION		
DIMENSION			GATE HEIGI	HTS "H" (ft	)	
DESIGNATIONS	6	8	10	12	14	16
L	7	9	9	12	12	15
В	6	6	6	9	9	10
Α	2.5	2.5	2.5	3	3	4
С	2	2	2.5	3	3	3.5
Т	1	1	1	1.5	1.5	1.75
GATE OPENING W	IDTH "W"=	10 ft				
	PILE DIN	<b>MENSIONS</b>	AND INFO	RMATION		
PILE TYPE		(	GATE HEIGI	HTS "H" (ft	)	
14 H-PILE	6	8	10	12	14	16
# Piles	4	6	6	9	9	15
Tip Elevation (ft)	-35	-40	-45	-50	-55	-55
Pile Length(ft)	37	43	48	53	58	58
# Rows	2	3	3	3	3	5
# Columns	2	2	2	3	3	3
S <sub>ROW.</sub> (ft)	4	3	3	4.5	4.5	3
S <sub>COL</sub> . (ft)	3	3	3	3	3	3.5

\C	ONCRETE I	MONOLITH	IDIMENSIC	ON				
DIMENSION		GATE HEIGHTS "H" (ft)						
DESIGNATIONS (ft)	5	7	9	11	13			
L	7	9	9	9	10			
В	6	6	6	6	9			
Α	2.5	2.5	2.5	2.5	3			
С	2	2	2	2.5	2.5			
Т	1	1	1	1.5	1.5			
<b>GATE OPENING WI</b>	="W" HTC	20 ft						
PIL	E DIMENSI	ONS AND I	NFORMAT	ION				
PILE TYPE		GATE HEIGHTS "H" (ft)						
14 H-PILE	5	7	9	11	13			
# Piles	4	6	6	6	9			
Tip Elevation (ft)	-35	-40	-50	-55	-55			
Pile Length(ft)	37	43	53	58	58			
# Rows	2	3	3	3	3			
# Columns	2	2	2	2	3			
S <sub>ROW.</sub> (ft)	4	3	3	3	3.5			
S <sub>COL</sub> . (ft)	3	3	3	3	3			
GATE OPENING WIL	TH "W"=	20 ft						

	T	S	WING GA	TE STEEL M	FINIREK 212	LES					
GATE					GATE HEIG	HTS "H" (ft	)				
DESIGNATIONS	2*	4	6	8	10	12	13	14	16	18	
Top Girder	W12x16	W12x16	W12x22	W16x26	W18x35	W24x55	W24x55	W24x55	W24x62	W24	
Bottom Girder	W12x26	W12x26	W12x35	W16x50	W18x65	W24x84	W24x84	W24x94	W24x117	W24x	
Intercostal	WT3x6	WT3x6	WT3x6	WT3x6	WT4x6.5	WT5x7.5	WT5x7.5	WT6x8	WT7x11	WT7	
X-Bracing	C10x20	C10x20	C10x20	C10x20	C10x20	C10x20	C10x20	C10x20	C10x20	C10>	
Skin Plate	5/16	5/16	5/16	5/16	5/16	5/16	5/16	3/8	3/8	3/	
Horiz. Brace	L4x4x0.25	L4x4x0.25	L4x4x0.25	L4x4x0.5	L4x4x0.5	L4x4x0.5	L4x4x0.5	L4x4x0.5	L4x4x0.5	L4x4	
GATE OPENING WI	DTH "W"=	30 ft									
* Gate Height was r	ounded up	to 2ft									
			CONC	RETE MON	OLITH DIM	ENSION					
DIMENSION		GATE HEIGHTS "H" (ft)									
DESIGNATIONS (ft)	2*	4	6	8	10	12	13	14	16	1	
L	6	6	9	9	10	12	12	12	15	1	
В	6	6	6	6	9	9	9	9	10	1	
А	2.5	2.5	2.5	2.5	3	3	3	3	4	4	
С	1.5	1.5	2	2	2.5	3	3	3	3.5	4	
Т	1	1	1	1	1	1.5	1.5	1.5	1.75	2	
GATE OPENING WI	DTH "W"=	30 ft									
* Gate Height was r	ounded up	to 2ft									
			PILE DIN	<b>IENSIONS</b>	AND INFO	RMATION					
PILE TYPE					GATE HEIG	HTS "H" (ft	)				
14 H-PILE	2*	4	6	8	10	12	13	14	16	1	
# Piles	4	4	6	6	9	9	9	9	15	1.	
Tip Elevation (ft)	-40	-40	-40	-50	-50	-55	-60	-60	-60	-6	
Pile Length(ft)	43	43	43	53	53	58	64	64	64	6	
# Rows	2	2	3	3	3	3	3	3	5	5	
# Columns	2	2	2	2	3	3	3	3	3	3	
S <sub>ROW.</sub> (ft)	3	3	3	3	3.5	4.5	4.5	4.5	3	3.	
S <sub>COL</sub> . (ft)	3	3	3	3	3	3	3	3	3.5	3.	
GATE OPENING WI	DTH "W"=	30 ft									
* Gate Height was r											

S	WING GAT	TE STEEL M	EMBER SIZ	ES	
GATE		GATE	HEIGHTS "	H" (ft)	
DESIGNATIONS	9	11	13	15	17
Top Girder	W18x35	W24x55	W24x55	W24x68	W27x84
Bottom Girder	W18x65	W24x76	W24x103	W24x131	W27x146
Intercostal	WT4x6.5	WT5x7.5	WT6x8.0	WT7x11	WT7x17
X-Bracing	C10x20	C10x20	C10x20	C10x20	C10x20
Skin Plate	5/16	5/16	5/16	3/8	3/8
Horiz. Brace	L4x4x0.25	L4x4x0.5	L4x4x0.5	L4x4x0.5	L4x4x0.5
GATE OPENING WII	DTH "W"=	35 ft			
C	ONCRETE I	MONOLITH	DIMENSIC	ON	
DIMENSION		GATE	HEIGHTS "	H" (ft)	
DESIGNATIONS (ft)	9	11	13	15	17
L	10	12	12	15	17
В	9	9	9	10	10
А	3	3	3	4	4
С	2.5	3	3	3.5	4
Т	1	1.5	1.5	1.75	2
GATE OPENING WII	DTH "W"=	35 ft			
PIL	E DIMENSI	ONS AND I	NFORMAT	ION	
PILE TYPE		GATE	HEIGHTS "	H" (ft)	
14 H-PILE	9	11	13	15	17
# Piles	9	9	9	15	15
Tip Elevation (ft)	-50	-55	-60	-60	-60
Pile Length(ft)	53	58	64	64	64
# Rows	3	3	3	5	5
# Columns	3	3	3	3	3
S <sub>ROW.</sub> (ft)	3.5	4.5	4.5	3	3.5
S <sub>COL</sub> . (ft)	3	3	3	3.5	3.5
GATE OPENING WII	DTI I II MATII	35 ft			

	of	S A En	gin	eer	s		
							APPR.

_				
				APPR.
				DATE
				DESCRIPTION
				MARK
				APPR.
				DATE APPR. MARK
				DESCRIPTION
				MARK

)	SWING GA	TE STEEL M	EMBER SIZ	ES
,	GATE	GATE	HEIGHTS "	H" (ft)
	DESIGNATIONS	10	12	14
	Top Girder	W24x55	W24x62	W24x76
	Bottom Girder	W24x84	W24x117	W24x162
	Intercostal	WT5x7.5	WT6x8.0	WT7x11
	X-Bracing	C10x20	C10x20	C10x20
	Skin Plate	5/16	5/16	3/8
	Horiz. Brace	L4x4x0.25	L4x4x0.5	L4x4x0.5
	<b>GATE OPENING WII</b>	DTH "W"=	40 ft	
	CONCRETE	MONOLITH	I DIMENSI	ON
	DIMENSION	GATE	HEIGHTS "	H" (ft)
	DESIGNATIONS (ft)	10	12	14
	L	12	12	15
	В	9	9	10
	Α	3	3	4
	С	3	3	3.5
	Т	1.5	1.5	1.75
	GATE OPENING WII	DTH "W"=	40 ft	
	PILE DIMENS	IONS AND	INFORMAT	TION
	PILE TYPE	GATE	HEIGHTS "	H" (ft)
	14 H-PILE	10	12	14
	# Piles	9	12	15
	Tip Elevation (ft)	-55	-60	-60
	Pile Length(ft)	58	64	64
	# Rows	3	4	5
	# Columns	3	3	3
	S (f+)	15	3	2

GATE OPENING WIDTH "W"= 40 ft

SWING GA	IE STEEL IVI	EIVIBER SIZ	.ES	
GATE	GATE	HEIGHTS "	H" (ft)	
DESIGNATIONS	5	7	9	
Top Girder	W16x26	W21x44	W21x48	
Bottom Girder	W16x45	W21x62	W21x93	
Intercostal	WT3x6	WT3x6	WT3x6	
X-Bracing	C10x20	C10x20	C10x20	
Skin Plate	5/16	5/16	5/16	
Horiz. Brace	L4x4x0.25	L4x4x0.5	L4x4x0.5	
<b>GATE OPENING WII</b>	DTH "W"=	45 ft		
CONCRETE	MONOLITH	I DIMENSI	ON	
DIMENSION	GATE	HEIGHTS "	H" (ft)	
DESIGNATIONS (ft)	5	7	9	
L	9	10	12	
В	6	9	9	
А	3	3	3	
С	2.5	3	3	
Т	1.5	1.5	1.5	
<b>GATE OPENING WII</b>	DTH "W"=	45 ft		
PILE DIMENS	IONS AND	INFORMAT	ΓΙΟΝ	
PILE TYPE	GATE	HEIGHTS "	H" (ft)	
14 H-PILE	5	7	9	
# Piles	6	9	9	
Tip Elevation (ft)	-45	-45	-50	
Pile Length(ft)	48	48	53	
# Rows	3	3	3	
# Columns	2	3	3	
S <sub>ROW.</sub> (ft)	3	3.5	4.5	
S <sub>COL</sub> . (ft)	3	3	3	
GATE OPENING WII	TH "W"=	45 ft		

SWING GA	SWING GATE STEEL MEMBER SIZES			
GATE	GATE HEIGHTS "H" (ft			
DESIGNATIONS	6	8	10	
Top Girder	W18x40	W24x55	W24x62	
Bottom Girder	W18x65	W24x84	W24x131	
Intercostal	WT3x6	WT3x6	WT3x6	
X-Bracing	C10x20	C10x20	C10x20	
Skin Plate	5/16	5/16	5/16	
Horiz. Brace	L4x4x0.25	L4x4x0.5	L4x4x0.5	
GATE OPENING WIE	="W" HTC	50 ft		
CONCRETE I	CONCRETE MONOLITH DIMENSION			
DIMENSION	DIMENSION GATE HEIGHTS "H" (ft)			
DESIGNATIONS (ft)	6	8	10	
L	9	10	12	
В	6	9	9	
А	3	3	3	
С	2.5	3	3	
Т	1.5	1.5	1.5	
GATE OPENING WIE	="W" HTC	50 ft		
PILE DIMENSI	IONS AND	INFORMAT	ΓΙΟΝ	
PILE TYPE	GATE	HEIGHTS "H" (ft)		
14 H-PILE	6	8	10	
# Piles	6	9	12	
Tip Elevation (ft)	-50			
Pile Length(ft)	53	53	58	
# Rows	3	3	4	
# Columns	2	3	3	
S <sub>ROW</sub> . (ft)	3	3.5	3	
S <sub>COL</sub> . (ft)	3	3	3	
GATE OPENING WIE	TH "W"=	50 ft		

1)	SWING GATE STEEL MEMBER SIZES					
' /	GATE	GATE HEIGHTS "H" (ft)				
	DESIGNATIONS	11	13	15	17	
	Top Girder	W21x44	W21x44	W24x55	W24x68	
	Bottom Girder	W21x68	W21x93	W24x104	W24x146	
	Intercostal	WT5x7.5	WT6x8.0	WT7x17	WT7x17	
	X-Bracing	C10x20	C10x20	C10x20	C10x20	
	Skin Plate	5/16	5/16	3/8	3/8	
	Horiz. Brace	L4x4x0.25	L4x4x0.5	L4x4x0.5	L4x4x0.5	
	<b>GATE OPENING WII</b>	DTH "W"=	30 ft			
	CONCI	RETE MON	OLITH DIM	ENSION		
	DIMENSION	IMENSION GATE HEIGHTS "H" (ft)				
	DESIGNATIONS (ft)	11	1 13 15			
	L	12	12	15	17	
	В	9	9	10	10	
	А	3	3	4	4	
	С	3	3	3.5	4	
	Т	1.5	1.5	1.75	2	
	<b>GATE OPENING WII</b>	DTH "W"=	30 ft			
	DII F DIM	IENSIONS A	AND INFOR	RMΔTI∩N		

HARRISON AREA (MICROPILES)

1.5	1.5	1.75					
="W" HTC	30 ft						
PILE DIMENSIONS AND INFORMATION							
GATE HEIGHTS "H" (ft)							
11	13	15	17				
9	9	12	15				
-60	-60	-60	-60				
64	64	64	64				
3	3	4	5				
3	3	3	3				
4.5	4.5	4	3.5				
3	3	3.5	3.5				
GATE OPENING WIDTH "W"= 30 ft							
CLUDES 10'	SOCKET, A	SSUMED					
BED ROCK @ EL50.0 SOCKET TIP @ EL60.0							
	11 9 -60 64 3 3 4.5 3 OTH "W"=	SOUTH "W" =   30 ft   30 ft	STH "W" =   30 ft       30 ft     30 ft     30 ft     30 ft     30 ft     30 ft     30 ft     30 ft     30 ft     30 ft       30 ft				

		SWING GAT	E STEEL MEM	BER SIZES		
GATE		GATE HEIGHTS "H" (ft)				
DESIGNATIONS	2	4	6	8	10	12
Top Girder	W12x19	W12x26	W12x30	W18x40	W24x62	W24x76
Bottom Girder	W12x35	W12x35	W12x53	W18x76	W24x94	W24x11
Intercostal	WT3x6	WT3x6	WT3x6	WT5x7.5	WT5x7.5	WT6x8.0
X-Bracing	C10x20	C10x20	C10x20	C10x20	C10x20	C10x20
Skin Plate	5/16	5/16	5/16	5/16	5/16	5/16
Horiz. Brace	L4x4x0.25	L4x4x0.25	L4x4x0.25	L4x4x0.25	L4x4x0.25	L4x4x0.2
GATE OPENING WIL	OTH "W"=	40 ft				
		CONCRETE N	ONOLITH DI	MENSION		
DIMENSION			GATE HE	IGHTS "H" (ft)		
DESIGNATIONS (ft)	2	4	6	8	10	12
L	6	6	9	9	12	12
В	6	6	6	6	6	6
Α	2.5	2.5	2.5	3	3	3
С	1.5	2	2	2.5	3	3
Т	1	1.5	1.5	1.5	1.5	1.5
GATE OPENING WIDTH "W"=		40 ft				
	P	ILE DIMENSION	ONS AND INF	ORMATION		
PILE TYPE	GATE HEIGHTS "H" (ft)					
9" MICROPILES	2	4	6	8	10	12
# Piles	4	4	6	6	8	8
Tip Elevation (ft)®	-60	-60	-60	-60	-60	-60
Pile Length(ft)	64	64	64	64	64	64
# Rows	2	2	3	3	4	4
# Columns	2	2	2	2	2	2
S <sub>ROW.</sub> (ft)	3	3	3	3	3	3
S <sub>COL</sub> . (ft)	3	3	3	3	3	3

